

GENERAL NOTES

Table with 2 columns: No. and NOTE. Contains 68 numbered notes regarding construction standards, safety, and procedures.

SYMBOLS

Table showing symbols for various architectural elements like orientation, grid lines, interior elevations, exterior elevations, building sections, wall sections, detail sections, call-outs, room designations, revisions, door and window symbols, glazed systems, wall types, floor plans, exit signs, furniture, exterior finishes, elevation datums, match lines, grade breaks, height, casework symbols, depth, wheel chair turning radius, and ADA clearances.

ABBREVIATIONS

Table with 2 columns: ABBREV. and DESCRIP. Lists abbreviations for materials, finishes, and construction elements such as anchor bolt, asphalt concrete, area drain, etc.

NOTES - ABBREVIATIONS

Table with 2 columns: ABBREV. and DESCRIP. Lists abbreviations for building components like machine bolt, medicine cabinet, masonry opening, etc.

8/29/2023 2:18:16 PM Autodesk Docs/Compton College Student Housing/Compton College Student Housing_AREV1

IDENTIFICATION STAMP with APP: 03-123205 INC, REVIEWED FOR, and DATE: 10/02/2023.



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o: 949.675.6442

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CONSULTANTS Design Collective ARCHITECTURE PLANNING INTERIORS LANDSCAPE ARCHITECTURE GRAPHICS

PROJECT TITLE COMPTON COLLEGE STUDENT HOUSING
INCREMENT 1 of 2 - DEMOLITION, EARTHWORK, & UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD, COMPTON, CA 90221



ISSUED table with columns #, DATE, and DESCRIPTION. Shows one issued item on 09/05/2023: DSA BACKCHECK SUBMITTAL.

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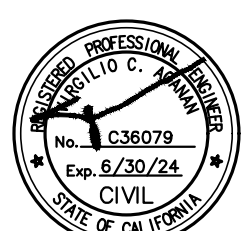
SHEET TITLE GENERAL NOTES, SYMBOLS, AND ABBREVIATIONS

SHEET NUMBER G1.20-01



A# 03-123205 INC: 01

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PROJECT TITLE
**COMPTON COLLEGE
STUDENT HOUSING**
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK &
UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD., COMPTON, CA 90221



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SHEET TITLE
**OVERALL SITE DEMOLITION
PLAN**

SHEET NUMBER

CD-1.0-01

REMOVAL NOTES:

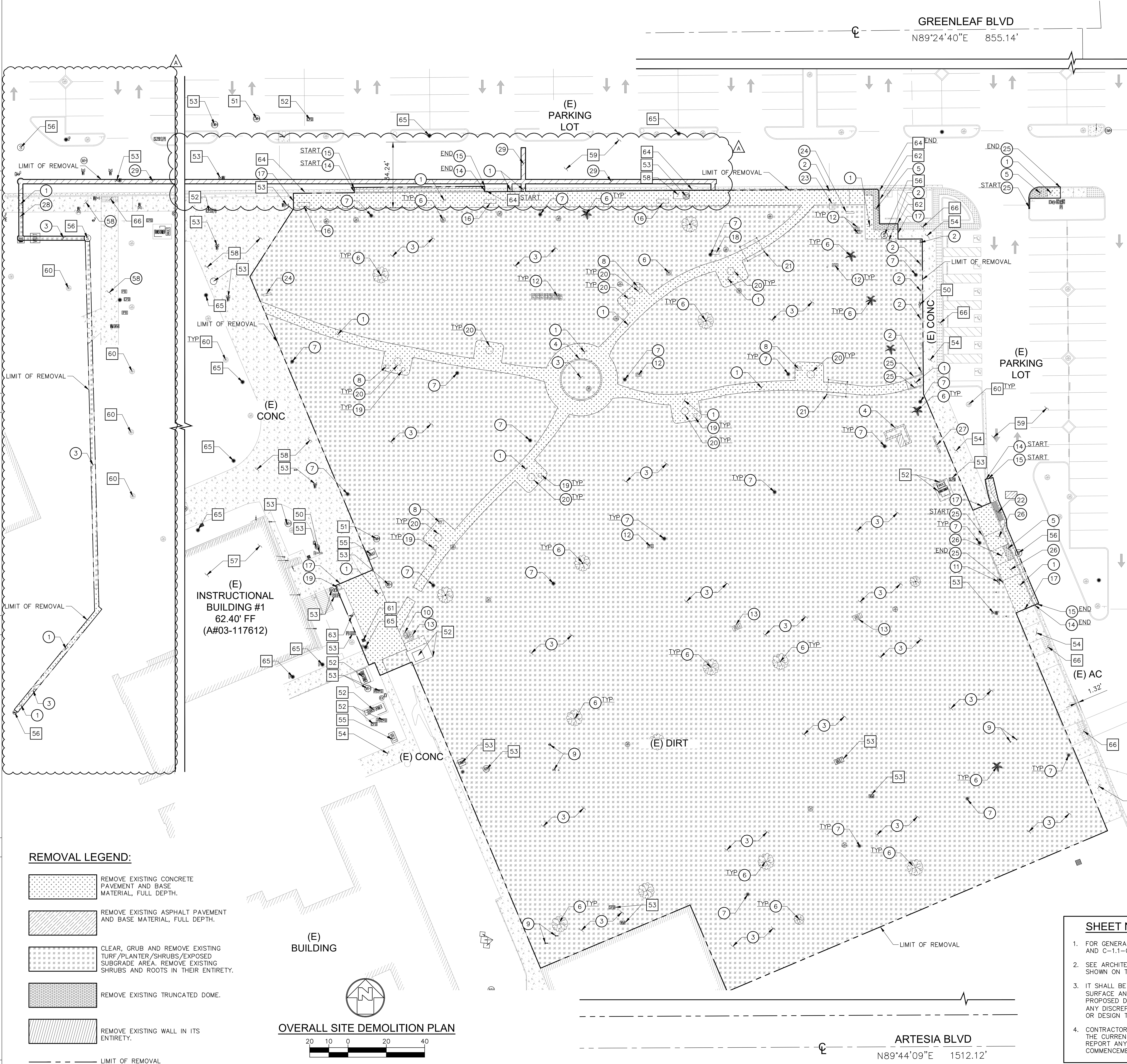
- 1 REMOVE EXISTING CONCRETE PAVEMENT AND BASE MATERIAL, FULL DEPTH.
- 2 REMOVE, SALVAGE AND REINSTALL EXISTING POST SIGN AT THE SAME LOCATION.
- 3 CLEAR, GRUB AND REMOVE EXISTING TURF/PLANTER/EXPOSED SUBGRADE AREA. REMOVE EXISTING SHRUBS AND ROOTS.
- 4 REMOVE EXISTING WALL/CURB WALL AND FOOTINGS IN ITS ENTIRETY.
- 5 REMOVE EXISTING TRUNCATED DOME IN ITS ENTIRETY.
- 6 REMOVE EXISTING TREES IN ITS ENTIRETY. COORDINATE WITH LANDSCAPE DRAWINGS AND COLLEGE PRIOR TO REMOVAL.
- 7 REMOVE EXISTING STREET LIGHT IN ITS ENTIRETY. CAP AND PLUG AS REQUIRED. COORDINATE WITH ELECTRICAL DRAWINGS.
- 8 REMOVE EXISTING DRINKING FOUNTAIN IN ITS ENTIRETY. CAP AND PLUG AS REQUIRED.
- 9 REMOVE EXISTING UTILITY PULL BOX, MANHOLE, WATER VALVE, OR CLEANOUTS.
- 10 REMOVE AND RELOCATE COMMUNICATION PULLBOX.
- 11 REMOVE AND RELOCATE EXISTING FIRE HYDRANT.
- 12 REMOVE EXISTING IRRIGATION PULLBOX. COORDINATE WITH LANDSCAPE DRAWINGS FOR RELOCATION.
- 13 REMOVE AND RELOCATE EXISTING VAULT.
- 14 REMOVE EXISTING CURB AND GUTTER IN ITS ENTIRETY.
- 15 SAWCUT AND JOIN ASPHALT PAVEMENT TO MATCH EXISTING.
- 16 REMOVE EXISTING BENCH. SALVAGE AND RETURN TO OWNER.
- 17 SAWCUT AND JOIN CONCRETE PAVEMENT/WALKWAY TO MATCH EXISTING.
- 18 REMOVE EXISTING STREET LIGHT PULLBOX.
- 19 REMOVE EXISTING TRASH CAN. SALVAGE AND RETURN TO OWNER.
- 20 REMOVE EXISTING TABLE. SALVAGE AND RETURN TO OWNER.
- 21 REMOVE EXISTING CANOPY AND ITS FOOTINGS IN ITS ENTIRETY. SALVAGE AND RETURN TO OWNER.
- 22 REMOVE, SALVAGE AND RELOCATE EXISTING STORM DRAIN INLET.
- 23 REMOVE EXISTING PAYBOOTH. SALVAGE AND RETURN TO OWNER.
- 24 REMOVE EXISTING DIRECTORY/POST SIGNS. SALVAGE AND RETURN TO OWNER.
- 25 REMOVE EXISTING CURB IN ITS ENTIRETY.
- 26 REMOVE EXISTING CURB RAMP.
- 27 REMOVE EXISTING DRAIN INLET.
- 28 REMOVE AND RELOCATE EMERGENCY UTILITY.
- 29 REMOVE EXISTING ASPHALT PAVEMENT AND BASE MATERIAL, FULL DEPTH.

PROTECT-IN-PLACE NOTES:

- 50 PROTECT IN PLACE FIRE HYDRANT. ADJUST TO NEW DESIGN GRADES AS REQUIRED.
- 51 PROTECT IN PLACE EXISTING COMMUNICATION MANHOLE.
- 52 PROTECT IN PLACE EXISTING ELECTRICAL PAD AND EQUIPMENT. COORDINATE WITH ELECTRICAL DRAWINGS FOR EXISTING ELECTRICAL ITEMS TO REMAIN.
- 53 PROTECT IN PLACE EXISTING UTILITY PULL BOX, MANHOLE, WATER VALVE, AND CLEANOUTS. ADJUST TO NEW DESIGN GRADES AS REQUIRED.
- 54 PROTECT IN PLACE EXISTING CONCRETE WALKWAY/SIDEWALK, AND BASE MATERIAL, FULL DEPTH.
- 55 PROTECT IN PLACE EXISTING ELECTRICAL VAULT.
- 56 PROTECT IN PLACE EXISTING SEWER MANHOLE.
- 57 PROTECT IN PLACE EXISTING BUILDING.
- 58 PROTECT IN PLACE EXISTING CONCRETE PAVEMENT AND BASE MATERIAL, FULL DEPTH.
- 59 PROTECT IN PLACE EXISTING AC PAVEMENT AND BASE MATERIAL, FULL DEPTH.
- 60 PROTECT IN PLACE EXISTING TREES.
- 61 PROTECT IN PLACE EXISTING STORM DRAIN INLET.
- 62 PROTECT IN PLACE EXISTING CURB.
- 63 PROTECT IN PLACE EXISTING GAS VALVE.
- 64 PROTECT IN PLACE EXISTING CURB AND GUTTER.
- 65 PROTECT IN PLACE EXISTING STREET LIGHT.
- 66 PROTECT IN PLACE EXISTING TRUNCATED DOME.

SHEET NOTES:

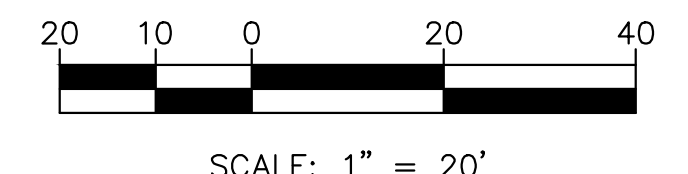
1. FOR GENERAL NOTES, LEGENDS AND ABBREVIATIONS, SEE SHEET C-1.0-01 AND C-1.1-01.
2. SEE ARCHITECTURAL DRAWINGS FOR OTHER SITE RELATED DIMENSIONS NOT SHOWN ON THIS DRAWING.
3. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY AND SURFACE AND/OR UNDERGROUND UTILITIES IN CONFLICT WITH THE PROPOSED DEMOLITION AND DESIGN ITEMS. CONTRACTOR SHALL REPORT ANY DISCREPANCIES AND/OR CONSTRUCTION RELATED ISSUES TO THE OWNER OR DESIGN TEAM PRIOR TO THE COMMENCEMENT OF WORK.
4. CONTRACTOR TO VERIFY IN FIELD THE JOINING TO EXISTING ELEVATION AND THE CURRENT SITE CONDITION WITH THE DESIGN GRADES. CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE OWNER OR DESIGN TEAM PRIOR TO THE COMMENCEMENT OF WORK.



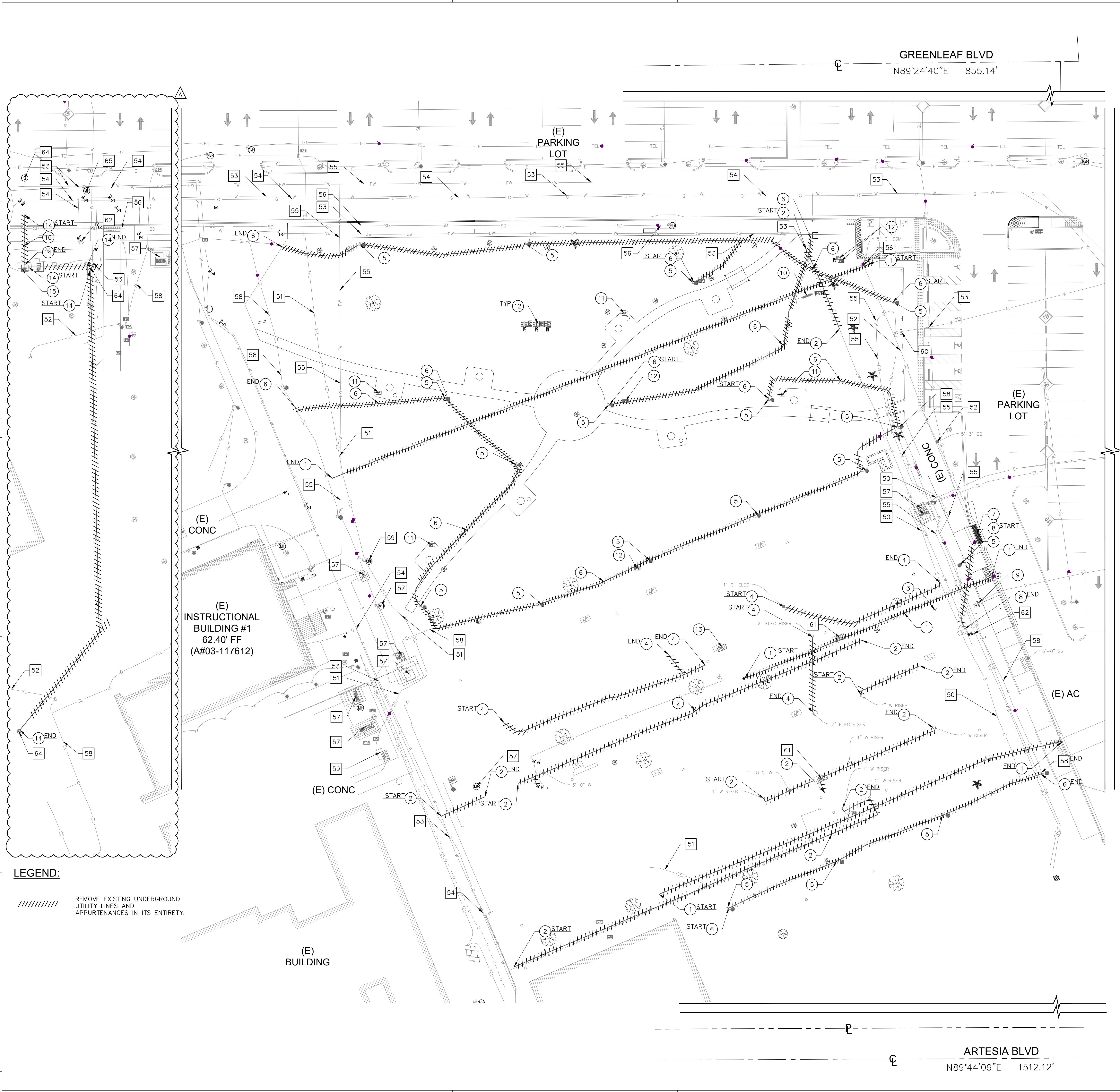
REMOVAL LEGEND:

- REMOVE EXISTING CONCRETE PAVEMENT AND BASE MATERIAL, FULL DEPTH.
- REMOVE EXISTING ASPHALT PAVEMENT AND BASE MATERIAL, FULL DEPTH.
- CLEAR, GRUB AND REMOVE EXISTING TURF/PLANTER/SHRUBS/EXPOSED SUBGRADE AREA. REMOVE EXISTING SHRUBS AND ROOTS IN THEIR ENTIRETY.
- REMOVE EXISTING TRUNCATED DOME.
- REMOVE EXISTING WALL IN ITS ENTIRETY.
- LIMIT OF REMOVAL

OVERALL SITE DEMOLITION PLAN



11/2/2022 5:02:26 PM IM 3407/Compton College Student Housing/Compton College Student Housing_A6.rvt



REMOVAL NOTES:

- 1 REMOVE EXISTING SEWER LINE AND APPURTENANCES IN ITS ENTIRETY. CAP AND PLUG BOTH ENDS AS REQUIRED.
- 2 REMOVE EXISTING WATER LINE IN ITS ENTIRETY. REFER TO PLUMBING DRAWINGS FOR MORE INFORMATION.
- 3 REMOVE EXISTING WATER VALVE.
- 4 REMOVE EXISTING ELECTRICAL LINE IN ITS ENTIRETY. REFER TO ELECTRICAL DRAWINGS FOR MORE INFORMATION.
- 5 REMOVE EXISTING STREET LIGHT IN ITS ENTIRETY. CAP AND PLUG AS REQUIRED. REFER TO ELECTRICAL DRAWINGS FOR MORE INFORMATION.
- 6 REMOVE EXISTING STREET LIGHT LINE IN ITS ENTIRETY. CAP AND PLUG AS REQUIRED. REFER TO ELECTRICAL DRAWINGS FOR MORE INFORMATION.
- 7 REMOVE AND RELOCATE EXISTING CATCH BASIN/DRAIN INLET AND APPURTENANCES IN ITS ENTIRETY.
- 8 REMOVE EXISTING STORM DRAIN LINE AND APPURTENANCES IN ITS ENTIRETY. CAP AND PLUG BOTH ENDS AS REQUIRED.
- 9 REMOVE AND RELOCATE EXISTING FIRE HYDRANT.
- 10 REMOVE AND RELOCATE EXISTING BACK FLOW PREVENTER.
- 11 REMOVE EXISTING DRINKING FOUNTAIN IN ITS ENTIRETY. CAP AND PLUG AS REQUIRED.
- 12 REMOVE EXISTING IRRIGATION PULLBOX.
- 13 REMOVE AND RELOCATE EXISTING VAULT.
- 14 REMOVE AND REPLACE EXISTING SEWER LINE AND APPURTENANCES IN ITS ENTIRETY. CAP AND PLUG BOTH ENDS AS REQUIRED.
- 15 REMOVE EXISTING SEWER EJECTOR AND APPURTENANCES IN ITS ENTIRETY. DISTRICT TO COORDINATE AND DISPOSED.
- 16 REMOVE AND RELOCATE EXISTING EMERGENCY POLE.

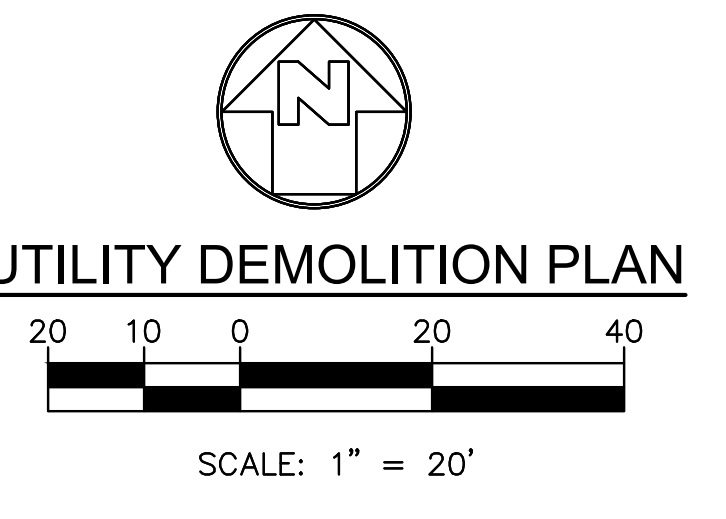
PROTECT-IN-PLACE NOTES:

- 50 PROTECT IN PLACE EXISTING ELECTRICAL LINE.
- 51 PROTECT IN PLACE EXISTING TELECOMMUNICATIONS LINE.
- 52 PROTECT IN PLACE EXISTING SEWER LINE AND BEND.
- 53 PROTECT IN PLACE EXISTING WATER LINE.
- 54 PROTECT IN PLACE EXISTING GAS LINE.
- 55 PROTECT IN PLACE EXISTING FIRE WATER LINE.
- 56 PROTECT IN PLACE EXISTING STORM DRAIN LINE.
- 57 PROTECT IN PLACE EXISTING ELECTRICAL VAULT, TRANSFORMER, CONDUITS IN ITS ENTIRETY.
- 58 PROTECT IN PLACE EXISTING STREET LIGHT LINE.
- 59 PROTECT IN PLACE EXISTING TELECOMMUNICATION BOX/VAULT, MANHOLE CONDUITS IN ITS ENTIRETY.
- 60 PROTECT IN PLACE FIRE HYDRANT.
- 61 PROTECT IN PLACE VAULT.
- 62 PROTECT IN PLACE EXISTING WATER VALVE.
- 63 PROTECT IN PLACE EXISTING STORM DRAIN MANHOLE.
- 64 PROTECT IN PLACE EXISTING SANITARY SEWER MANHOLE.
- 65 PROTECT IN PLACE EXISTING WATER MANHOLE.

SHEET NOTES:

1. FOR GENERAL NOTES, LEGENDS AND ABBREVIATIONS, SEE SHEET C-1.0-01 AND C1.1-01.
2. SEE ARCHITECTURAL DRAWINGS FOR OTHER SITE RELATED DIMENSIONS NOT SHOWN ON THIS DRAWING.
3. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY AND SURFACE AND/OR UNDERGROUND UTILITIES IN CONFLICT WITH THE PROPOSED DEMOLITION AND DESIGN ITEMS. CONTRACTOR SHALL REPORT ANY DISCREPANCIES AND/OR CONSTRUCTION RELATED ISSUES TO THE OWNER OR DESIGN TEAM PRIOR TO THE COMMENCEMENT OF WORK.
4. CONTRACTOR TO VERIFY IN FIELD THE JOINING TO EXISTING ELEVATION AND THE CURRENT SITE CONDITION WITH THE DESIGN GRADES. CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE OWNER OR DESIGN TEAM PRIOR TO THE COMMENCEMENT OF WORK.

LEGEND:
 REMOVE EXISTING UNDERGROUND UTILITY LINES AND APPURTENANCES IN ITS ENTIRETY.



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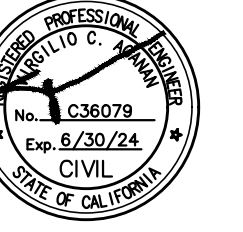
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PROJECT TITLE
COMPTON COLLEGE STUDENT HOUSING
 INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK & UNDERGROUND UTILITIES
 1111 E. ARTESIA BLVD., COMPTON, CA 90221



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SHEET TITLE
UTILITY DEMOLITION PLAN

SHEET NUMBER

CD-1.1-01

CONSTRUCTION DOCUMENTS

GREENLEAF BLVD
N89°24'40"E 855.14'

ESTIMATED EARTHWORK QUANTITY

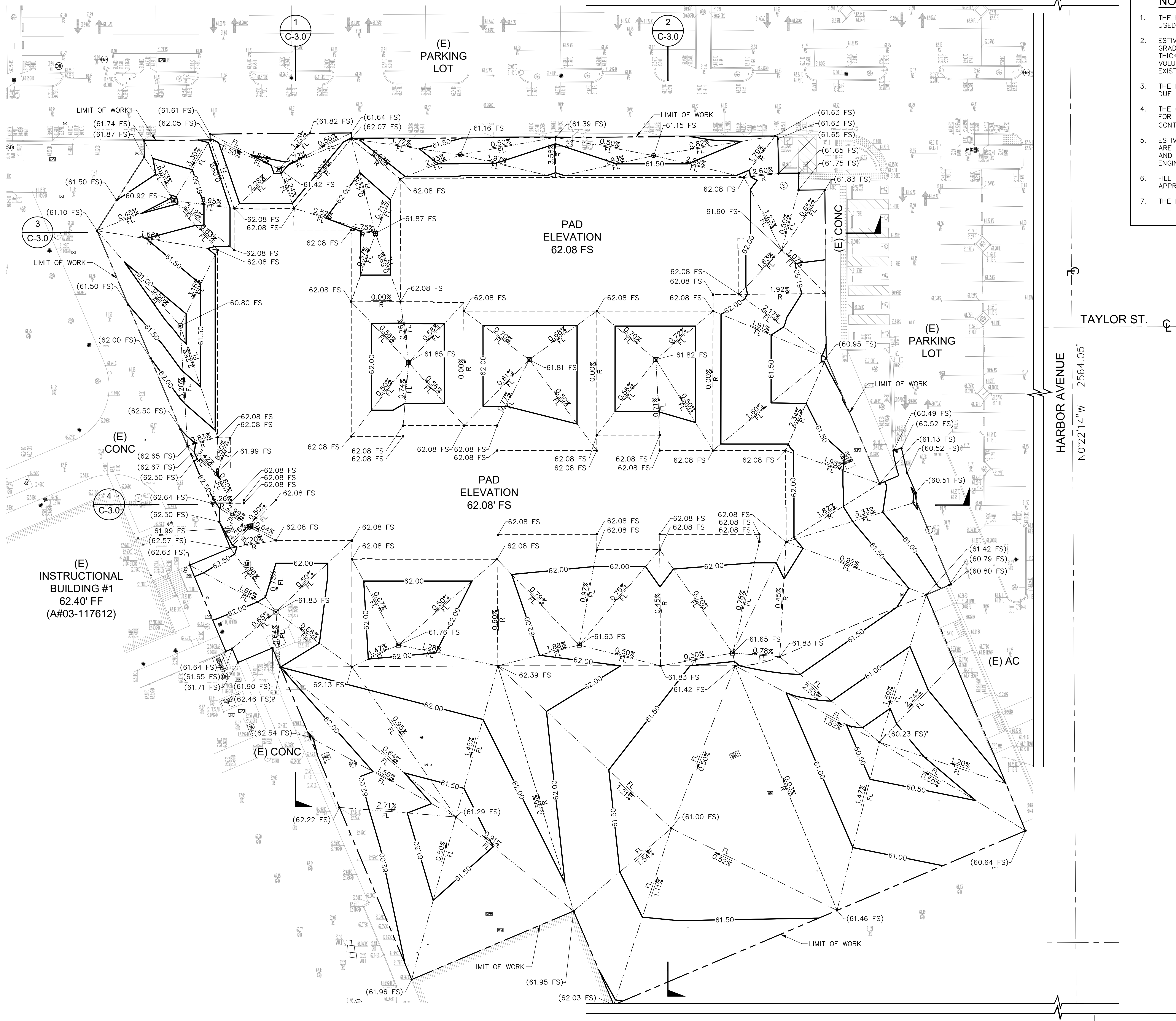
ESTIMATED CUT = 1225 CY
ESTIMATED FILL = 438 CY
ESTIMATED EXPORT = 787 CY

NOTES:

1. THE ESTIMATED QUANTITIES PROVIDED ABOVE ARE FOR REFERENCE ONLY TO BE USED FOR JURISDICTIONAL PLAN CHECKING AND PERMITTING PURPOSES ONLY.
2. ESTIMATED EARTHWORK ABOVE IS BASED ON DESIGN FINISH GRADES TO EXISTING GRADES IN SURVEY. THE ESTIMATED EARTHWORK DOES NOT CONSIDER THE THICKNESS OF EACH PAVEMENT MATERIAL, FOUNDATION AND SLAB ON GRADE VOLUMES, THE REMOVAL OF ANY UNSUITABLE MATERIAL, AND THE REMOVAL OF EXISTING BASEMENTS, PITS, VAULTS, TOP SOIL OR VEGETATION.
3. THE ESTIMATED EARTHWORK QUANTITIES DO NOT INCLUDE SHRINKAGE FACTORS DUE TO COMPACTION OR ANY OVER EXCAVATION QUANTITIES.
4. THE CONTRACTOR SHALL CALCULATE HIS OWN EARTHWORK QUANTITIES NECESSARY FOR HIS BID AND WORK. VCA IS NOT RESPONSIBLE AND LIABLE FOR THE CONTRACTOR'S EARTHWORK CALCULATIONS.
5. ESTIMATED EARTHWORK QUANTITIES ABOVE ASSUME THAT ALL ON-SITE MATERIALS ARE SUITABLE FOR BACKFILL. HOWEVER, ACTUAL EXISTING ON-SITE MATERIALS AND IMPORTED MATERIALS MUST FIRST BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO INSTALLATION, REMOVAL, OR REPLACEMENT.
6. FILL MATERIAL SHALL COMPLY WITH THE GEOTECHNICAL INVESTIGATION, AND BE APPROVED BY THE GEOTECHNICAL ENGINEER.
7. THE ESTIMATED EARTHWORK QUANTITIES DO NOT INCLUDE OVEREXCAVATION.

SHEET NOTES:

1. FOR GENERAL NOTES, LEGEND AND ABBREVIATIONS, SEE SHEET C-1.0-01 AND C-1.1-01.
2. SEE ARCHITECTURAL DRAWINGS FOR OTHER SITE RELATED DIMENSIONS NOT SHOWN ON THIS DRAWING.
3. FOR PAVEMENT MARKINGS AND STRIPING, RAMP WITH HANDRAILS, PLANTER AREA, TRASH ENCLOSURE AND TRANSFORMER ENCLOSURE, SEE ARCHITECTURAL DRAWINGS. FOR LANDSCAPING AND IRRIGATION FEATURES, CONCRETE PAVEMENT PATTERN, TEXTURE AND COLOR INTEGRATION, SEE LANDSCAPE DRAWINGS. FOR GAS, SEWER, ROOF DRAINS, FIRE & DOMESTIC WATER LINES POINTS OF CONNECTION AT BUILDING, SEE MECHANICAL AND PLUMBING DRAWINGS OF THE BUILDING. FOR TRANSFORMER ENCLOSURE AND CONCRETE PAD, SEE ELECTRICAL DRAWINGS.
4. FOR CATCH BASIN LOCATIONS AND GRATE ELEVATIONS AND INVERTS SEE UTILITY PLAN. IF TOP OF GRATE ELEVATIONS ARE DIFFERENT FROM GRADING CATCH BASIN ELEVATIONS SHOWN ON THIS SHEET, NOTIFY CPM AND ENGINEER.
5. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL SURFACE AND/OR UNDERGROUND UTILITIES IN CONFLICT WITH THE PROPOSED DEMOLITION AND DESIGN ITEMS. CONTRACTOR SHALL REPORT ANY DISCREPANCIES AND/OR CONSTRUCTION RELATED ISSUES TO THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
6. CONTRACTOR TO VERIFY IN FIELD THE JOINING TO EXISTING ELEVATION AND THE CURRENT SITE CONDITION WITH THE DESIGN GRADES. CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
7. CONTRACTOR SHALL VERIFY DEMOLITION PLAN WITH EXISTING CONDITIONS AND REPORT ANY MISSING ITEMS THAT REQUIRE DEMOLITION PRIOR TO CONSTRUCTION.



(E) INSTRUCTIONAL BUILDING #1
62.40' FF
(A#03-117612)

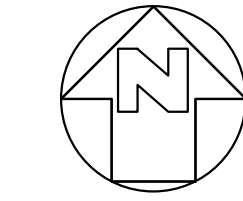
PAD
ELEVATION
62.08' FS

PAD
ELEVATION
62.08' FS

HARBOR AVENUE
NO°22'14"W 2564.05'

TAYLOR ST.

ARTESIA BLVD
N89°44'09"E 1512.12'



ROUGH GRADING PLAN



SCALE: 1" = 20'

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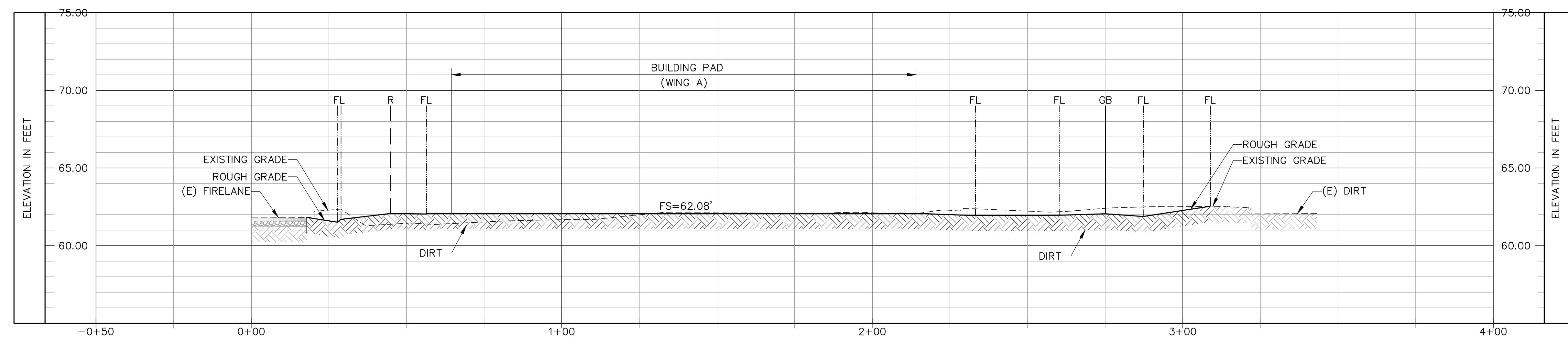
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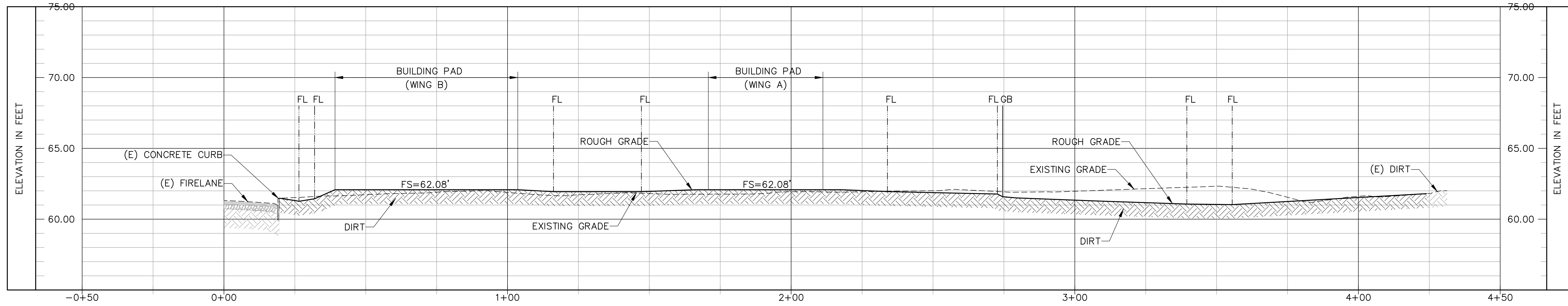
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ROUGH GRADING PLAN

SHEET NUMBER
C-3.0-01

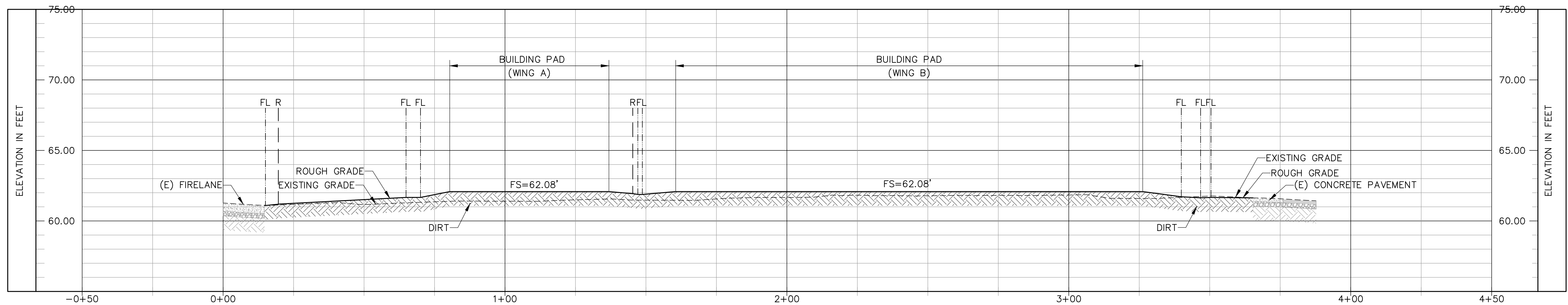
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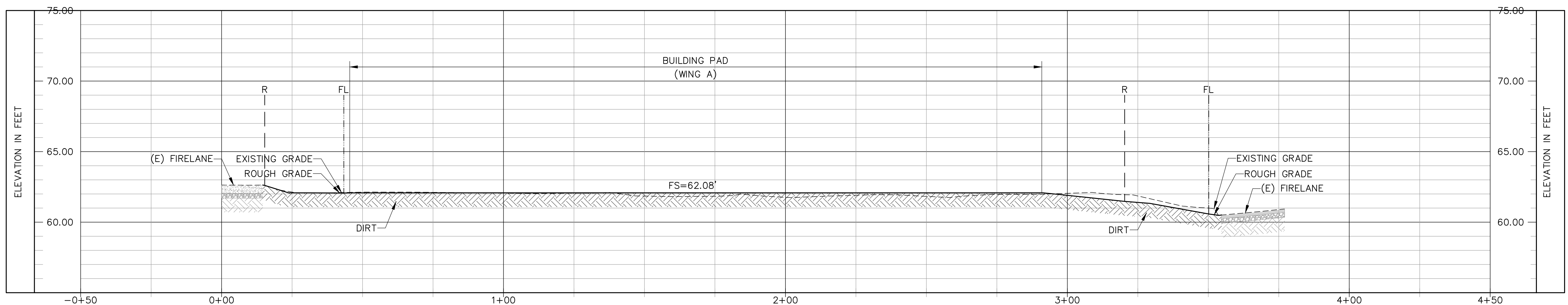
GRADING SECTION 1
SCALE HOR 1"=20' VER 1"=4' C-3.0



GRADING SECTION 2
SCALE HOR 1"=20' VER 1"=4' C-3.0



GRADING SECTION 3
SCALE HOR 1"=20' VER 1"=4' C-3.0

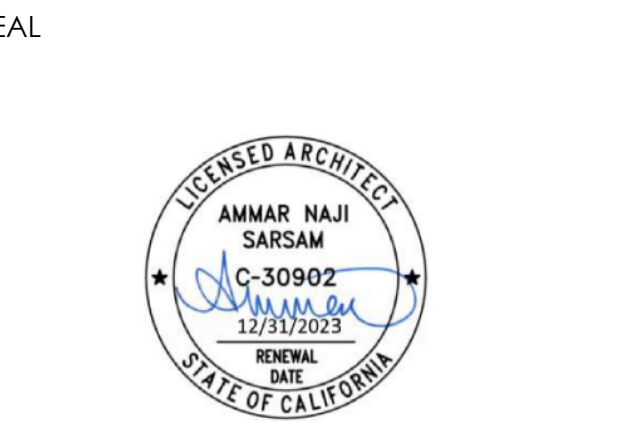


GRADING SECTION 4
SCALE HOR 1"=20' VER 1"=4' C-3.0

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SHEET TITLE
**ROUGH GRADING
 SECTIONS**

SHEET NUMBER
C-3.1-01



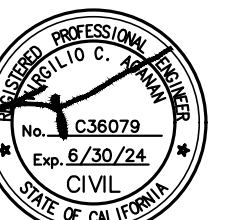
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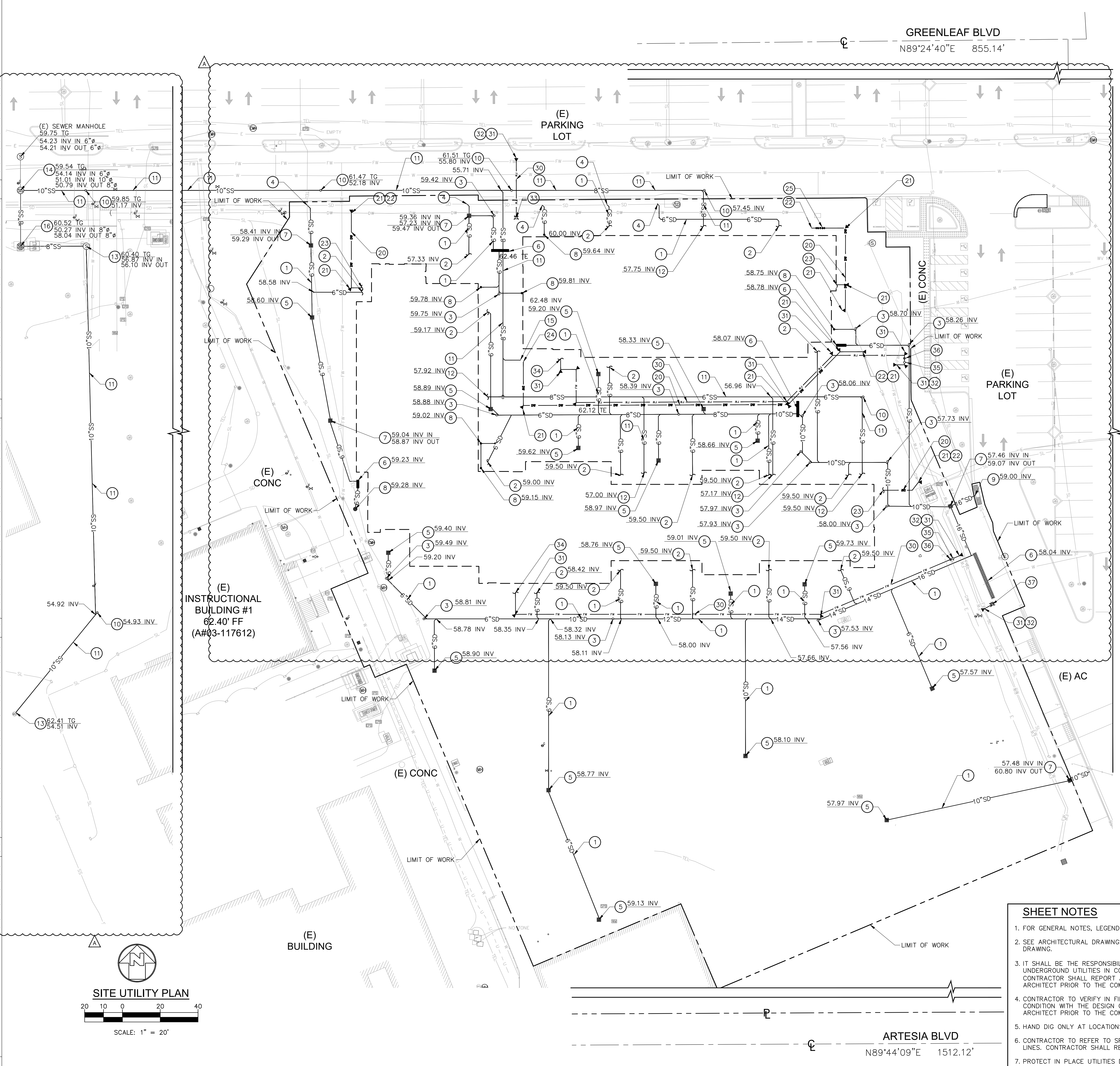
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SHEET TITLE
SITE UTILITY PLAN

SHEET NUMBER

C-4.0-01

CONSTRUCTION DOCUMENTS



CONSTRUCTION NOTES:

STORM DRAIN:

- INSTALL SDR-35 STORM DRAIN LINE, SEE PLAN FOR SIZES. FOR TRENCH SECTION REFER TO DETAIL 5 ON SHEET C-5.0-01.
- PROVIDE STUB OUT FOR FUTURE CONNECTION TO BUILDING LINE. CONNECTION TO BUILDING PER MEP DRAWINGS IN INC 2 PACKAGE.
- INSTALL STORMDRAIN CLEANOUT PER DETAIL 6 ON SHEET C-5.0-01.
- CONNECT TO EXISTING STORM DRAIN LINE. VERIFY IN FIELD EXACT LOCATION, INVERT AND DEPTH PRIOR TO CONSTRUCTION. COORDINATE WITH LA COUNTY FLOOD CONTROL DISTRICT FOR CONNECTION.
- CONSTRUCT CATCH BASIN WITH KRISTAR FILTER INSERT PER DETAIL 1 ON SHEET C-5.0-01.
- INSTALL TRENCH DRAIN PER DETAIL 3 ON SHEET C-5.1-01.
- CONSTRUCT BUBBLER CATCH BASIN PER DETAIL 2 ON SHEET C-5.1-01.
- CONSTRUCT AREA DRAIN PER DETAIL 4 ON SHEET C-5.1-01.
- CONNECT TO RELOCATED EXISTING CATCH BASIN/DRAIN INLET.

SANITARY SEWER:

- INSTALL SEWER CLEANOUT PER DETAIL 6 ON SHEET C-5.0-01.
- INSTALL PVC SCHEDULE 40 SANITARY SEWER LINE, SEE PLAN FOR SIZES. FOR TRENCH SECTION REFER TO DETAIL 5 ON SHEET C-5.0-01.
- PROVIDE STUB OUT FOR FUTURE CONNECTION TO BUILDING LINE. CONNECTION TO BUILDING PER MEP DRAWINGS IN INC 2 PACKAGE.
- CONNECT TO EXISTING ONSITE SANITARY SEWER LINE/MANHOLE. CONTRACTOR TO VERIFY SIZE IN FIELD AND PROVIDE REDUCING FITTINGS AND COUPLINGS AS NEEDED. CONTRACTOR TO VERIFY LOCATION IN FIELD PRIOR TO CONSTRUCTION/INSTALLATION OF SANITARY SEWER SYSTEM.
- INSTALL SEWER MANHOLE PER SPPWC STD. PLAN 200-4.
- CONNECT SEWER LINE TO DRINKING FOUNTAIN.
- INSTALL SEWER PUMP WITH GRINDER PER DETAIL 1 ON SHEET C-5.2-01.

DOMESTIC WATER:

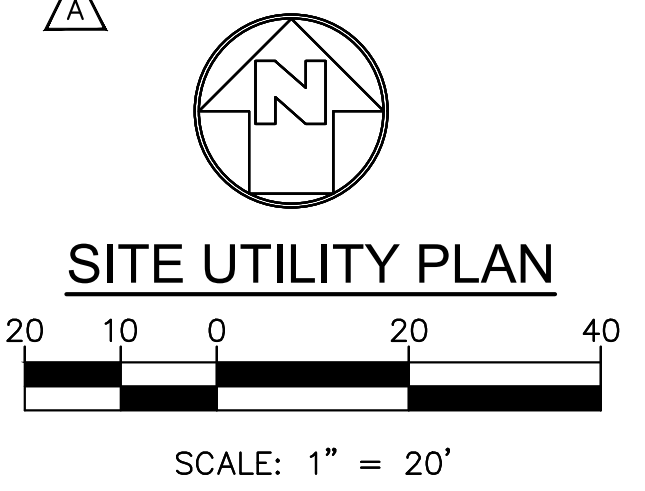
- INSTALL AWWA C900 PVC DOMESTIC WATER LINE, MATCH EXISTING PIPE SIZE. FOR TRENCH SECTION REFER TO DETAIL 5 ON SHEET C-5.0-01.
- INSTALL CONCRETE THRUST BLOCK PER DETAIL 7 ON SHEET C-5.0-01.
- CONNECT TO EXISTING ONSITE WATER LINE. CONTRACTOR TO VERIFY SIZE IN FIELD AND PROVIDE REDUCING FITTINGS AND COUPLINGS AS NEEDED. CONTRACTOR TO VERIFY LOCATION IN FIELD PRIOR TO CONSTRUCTION/INSTALLATION OF NEW DOMESTIC WATER SYSTEM.
- PROVIDE STUB OUT FOR FUTURE CONNECTION TO BUILDING LINE. CONNECTION TO BUILDING PER MEP DRAWINGS IN INC 2 PACKAGE.
- CONNECT WATER LINE TO DRINKING FOUNTAIN.
- REINSTALL EXISTING WATER BACKFLOW PREVENTER.

FIRE WATER:

- INSTALL AWWA C900 PVC, PRESSURE CLASS 200(DR14) FIRE WATER LINE. FOR TRENCH SECTION REFER TO DETAIL 5 ON SHEET C-5.0-01.
- INSTALL CONCRETE THRUST BLOCK PER DETAIL 7 ON SHEET C-5.0-01.
- CONNECT TO EXISTING ONSITE FIRE WATER LINE. CONTRACTOR TO VERIFY SIZE IN FIELD AND PROVIDE REDUCING FITTINGS AND COUPLINGS AS NEEDED. CONTRACTOR TO VERIFY LOCATION IN FIELD PRIOR TO CONSTRUCTION/INSTALLATION OF NEW FIRE WATER SYSTEM.
- INSTALL NEW FIRE HYDRANT PER DETAIL 2 ON SHEET C-5.0-01.
- PROVIDE STUB OUT FOR FUTURE CONNECTION TO BUILDING LINE. CONNECTION TO BUILDING PER FIRE PROTECTION DRAWINGS IN INC 2 PACKAGE.
- INSTALL FIRE DEPARTMENT CONNECTION PER DETAIL 5 ON SHEET C-5.1-01.
- INSTALL POST INDICATOR VALVE PER DETAIL 3 ON SHEET C-5.0-01.
- RELOCATED EXISTING FIRE HYDRANT.

SHEET NOTES

- FOR GENERAL NOTES, LEGENDS AND ABBREVIATIONS, SEE SHEET C-1.0-01 AND C-1.1-01.
- SEE ARCHITECTURAL DRAWINGS FOR OTHER SITE RELATED DIMENSIONS NOT SHOWN ON THIS DRAWING.
- IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL SURFACE AND/OR UNDERGROUND UTILITIES IN CONFLICT WITH THE PROPOSED DEMOLITION AND DESIGN ITEMS. CONTRACTOR SHALL REPORT ANY DISCREPANCIES AND/OR CONSTRUCTION RELATED ISSUES TO THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
- CONTRACTOR TO VERIFY IN FIELD THE JOINING TO EXISTING ELEVATION AND THE CURRENT SITE CONDITION WITH THE DESIGN GRADES. CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
- HAND DIG ONLY AT LOCATIONS WHERE UNDERGROUND UTILITIES NEED TO BE PROTECTED IN PLACE.
- CONTRACTOR TO REFER TO SPECIFICATIONS FOR THE REQUIRED COVER OF THE PROPOSED WATER LINES. CONTRACTOR SHALL REFER TO INCREMENT 2 GRADING PLAN FOR FINISH SURFACE ELEVATION.
- PROTECT IN PLACE UTILITIES DURING DEEP SOIL MIXING.



SITE UTILITY PLAN

SCALE: 1" = 20'

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SEAL



A# 03-123205 INC: 01

CONSULTANTS



Main table area containing 5 'Point Coordinate Table' sub-tables with columns for Point #, Northing, and Easting, listing coordinates for points 76 through 187.

PROJECT TITLE
COMPTON COLLEGE
STUDENT HOUSING
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK & UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD., COMPTON, CA 90221



ISSUED table with columns for #, DATE, and DESCRIPTION. Contains one entry: 03/01/2024 REVISION A.

PROJECT IDENTIFICATION
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SHEET TITLE
SITE UTILITY
COORDINATES PLAN

SHEET NUMBER

C-4.2-01

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PROJECT TITLE
**COMPTON COLLEGE
STUDENT HOUSING**
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK &
UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD., COMPTON, CA 90221



| ISSUED | | |
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| # | DATE | DESCRIPTION |
| Δ | 03/01/2024 | REVISION A |
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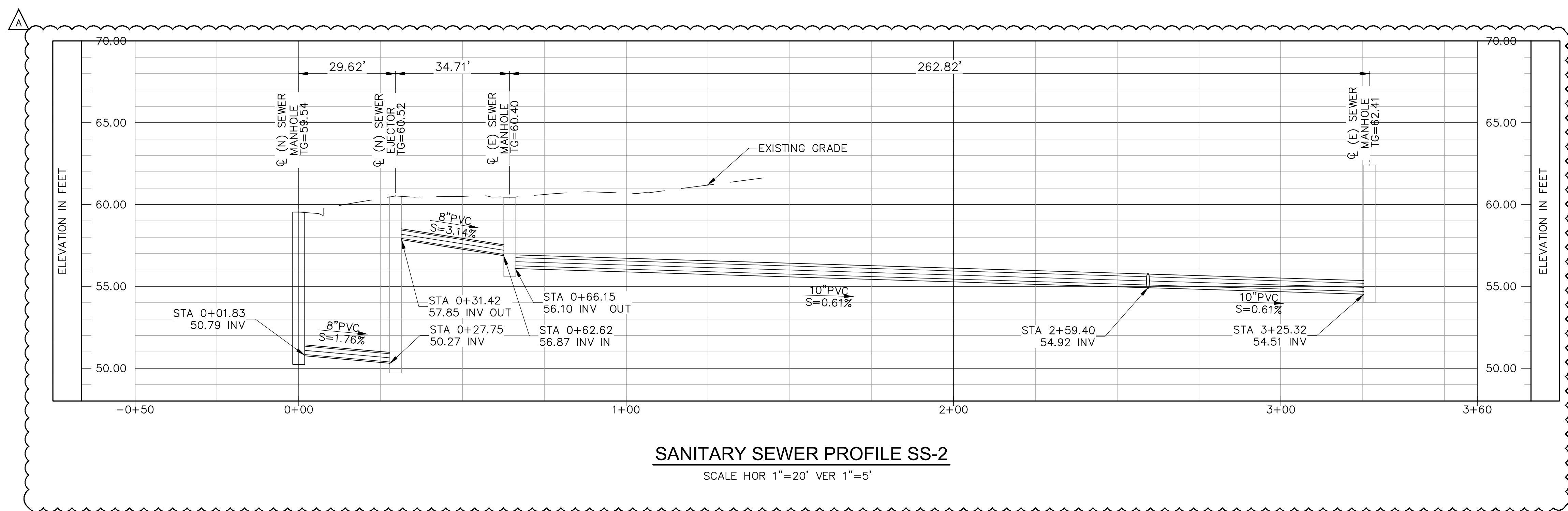
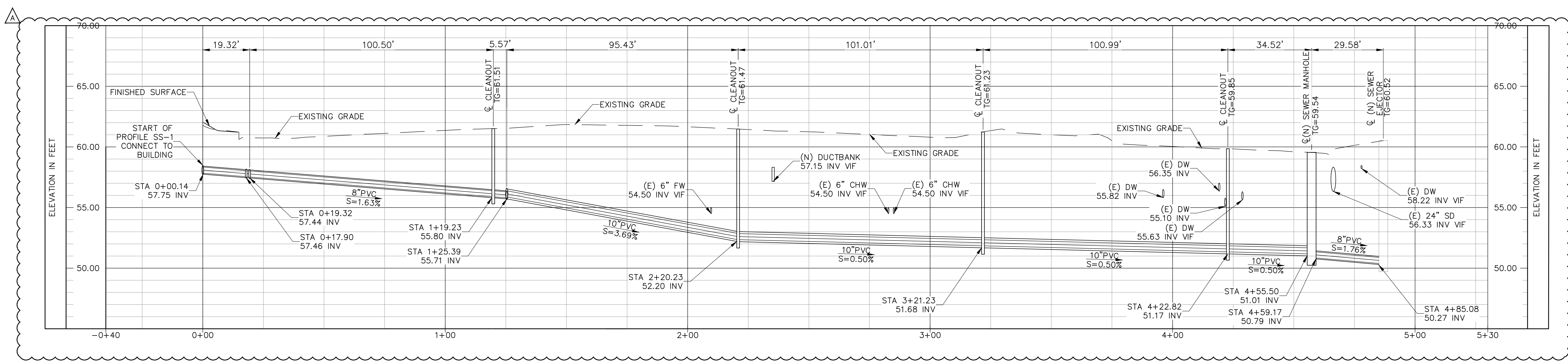
SHEET TITLE
SITE UTILITY PROFILE

SHEET NUMBER

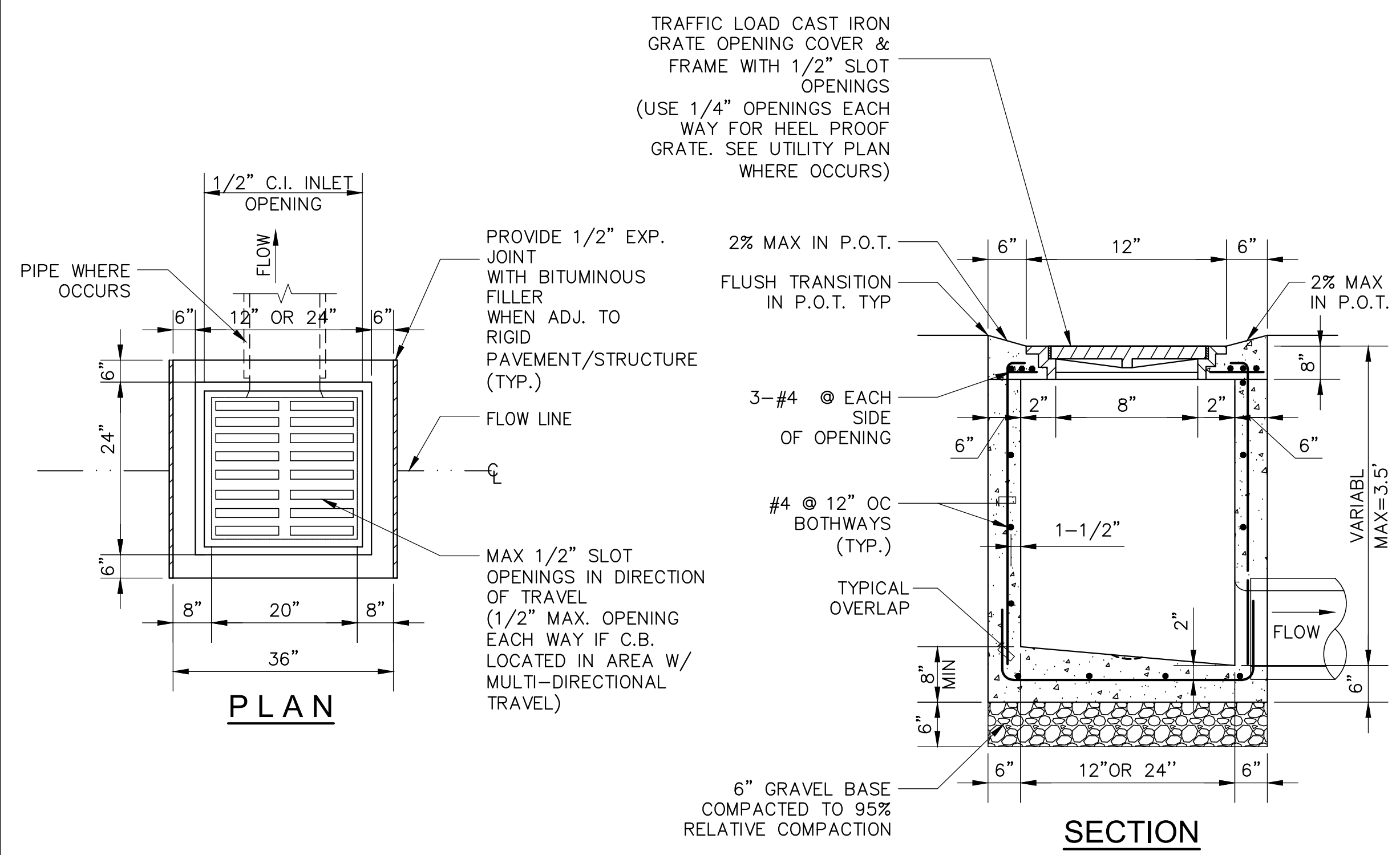
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CONSTRUCTION DOCUMENTS

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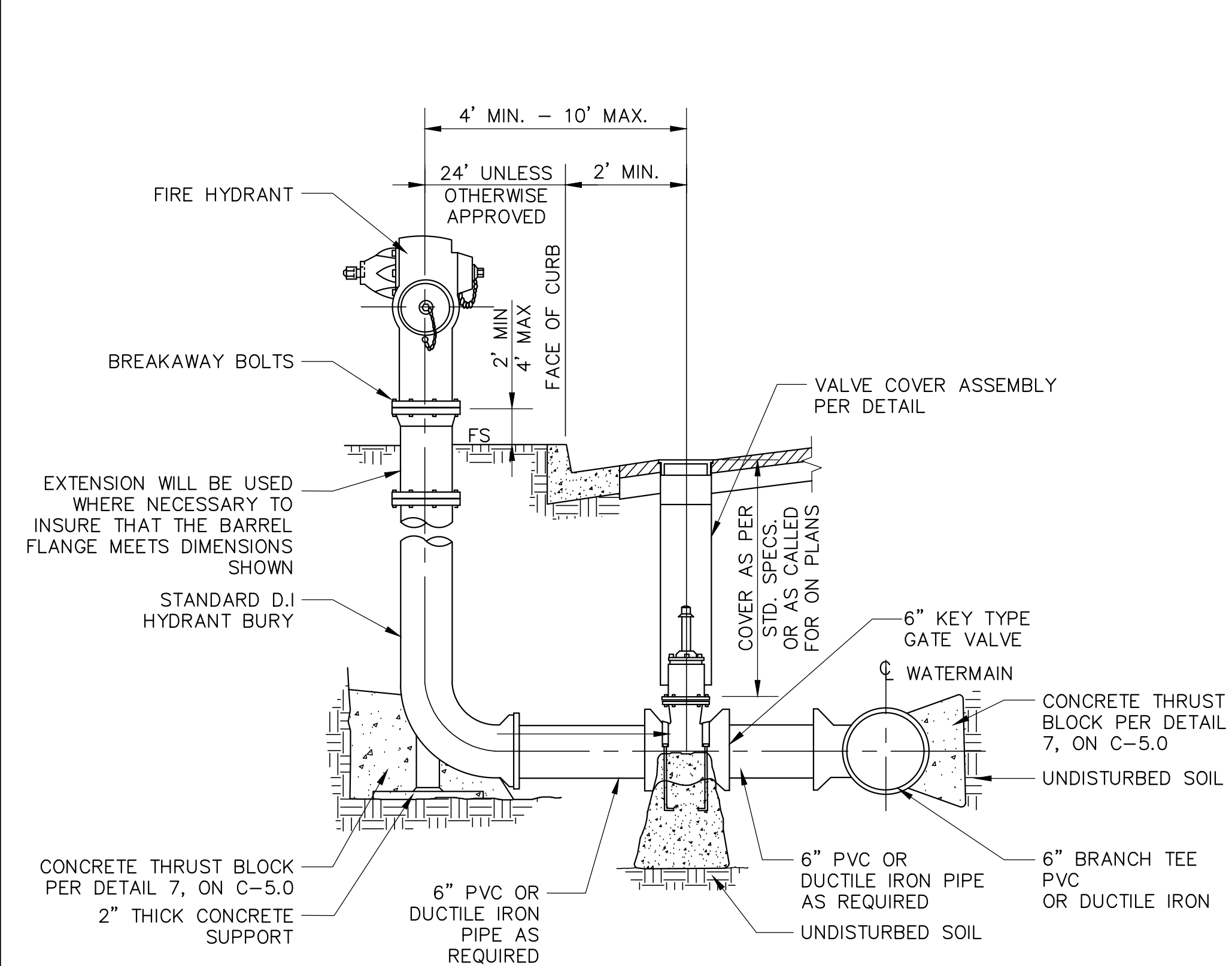


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- NOTES:**
- USE 3/4" DIA. PIPE BAR SPACERS ASSEMBLED ON (2) 1/2" DIA. RODS WITH THREADS AND NUTS AT BOTH ENDS.
 - ALL METAL PARTS SHALL BE GALVANIZED AFTER FABRICATION AND WELDING, AND BEFORE ASSEMBLING.
 - FRAME AND GRATE SHALL BE SIMILAR TO ALHAMBRA FOUNDRY CO. LTD. SERIES MODEL NO. 1581 OR BROOKS PRODUCTS, INC. OR APPROVED EQUAL. GRATES MUST COMPLY WITH ALL ADA REQUIREMENTS.
 - GRATES SHALL BE OF VANDAL-RESISTANT CONSTRUCTION WITH 1/2" MAX OPENINGS.
 - FRAME AND GRATE SHALL BE TRAFFIC-RATED.
 - GRATE MUST COMPLY WITH ADA REQUIREMENTS.
 - PROVIDE 1/2" MAX GRID/OPENINGS IN GRATING IN THE DIRECTION OF TRAFFIC FLOW UNLESS OTHERWISE NOTED HEREIN.
 - INSTALL FOSSIL FILTER, KRISTAR, (800) 579-8819, FLOGARD MODEL OR APPROVED EQUAL.
 - PROVIDE "NO DUMPING SYMBOL" PER DETAIL 4 ON SHEET C-5.0-01.
 - SLOPE ADJACENT PAVEMENT AT 2% MAX TOWARDS GRATE WHEN PLACED WITHIN ACCESSIBLE PATH OF TRAVEL PER ARCHITECTURAL DRAWINGS.
 - FOR COURTYARD AREA; PROVIDE 1/4" X 1/4" MAX GRID OPENINGS IN ALL DIRECTIONS IN GRATING.

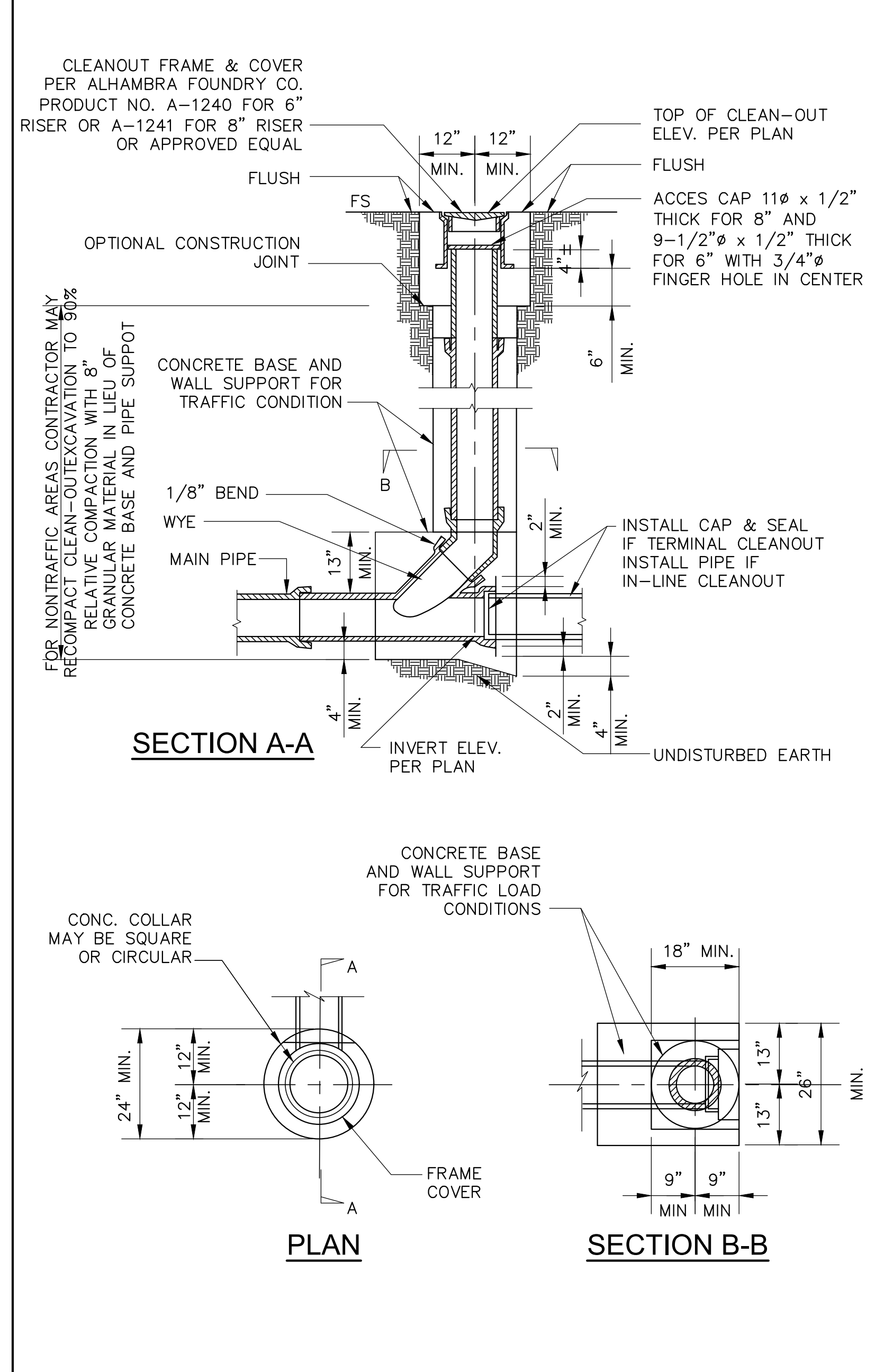
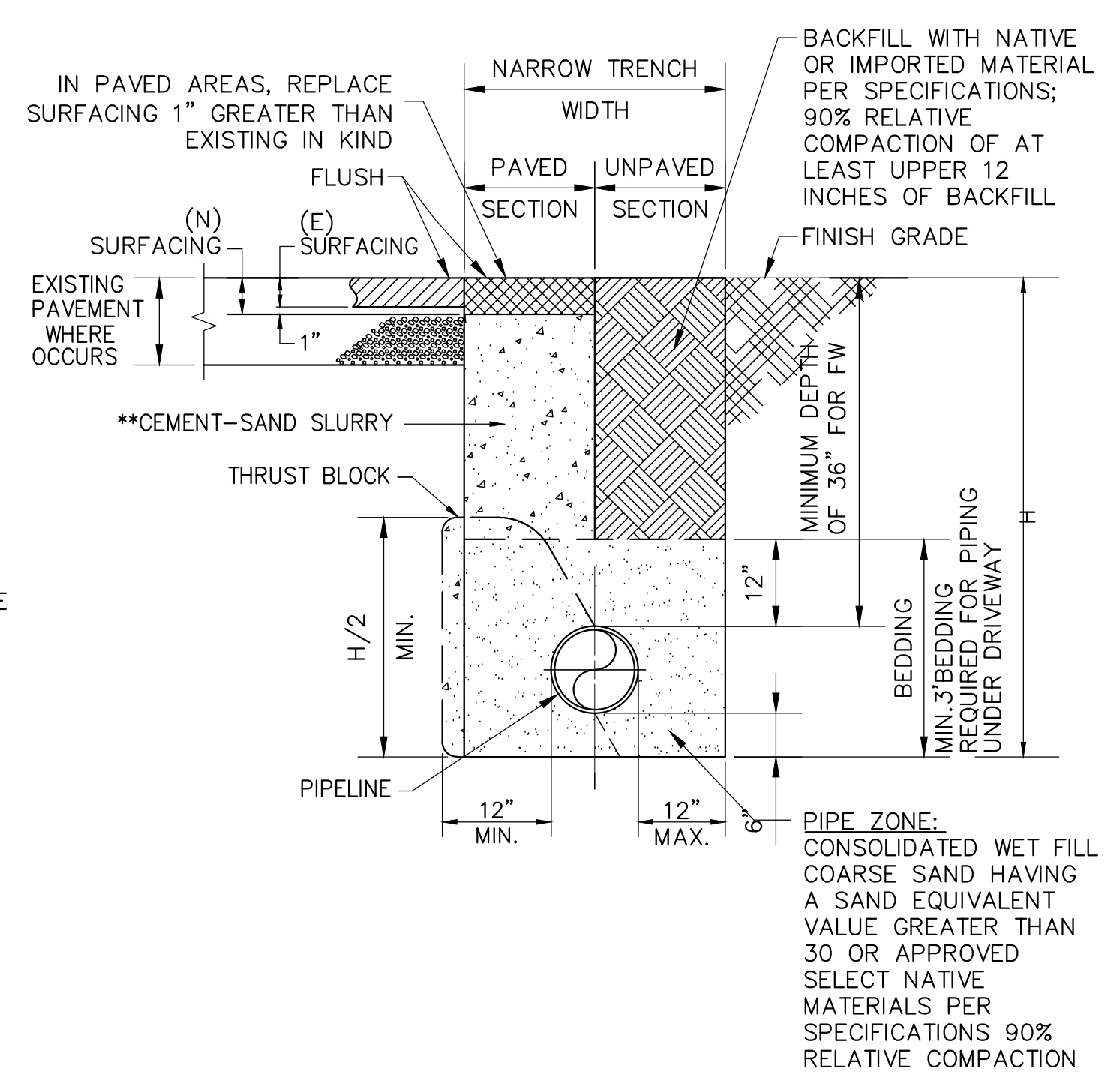
1 CATCH BASIN DETAIL
NOT TO SCALE



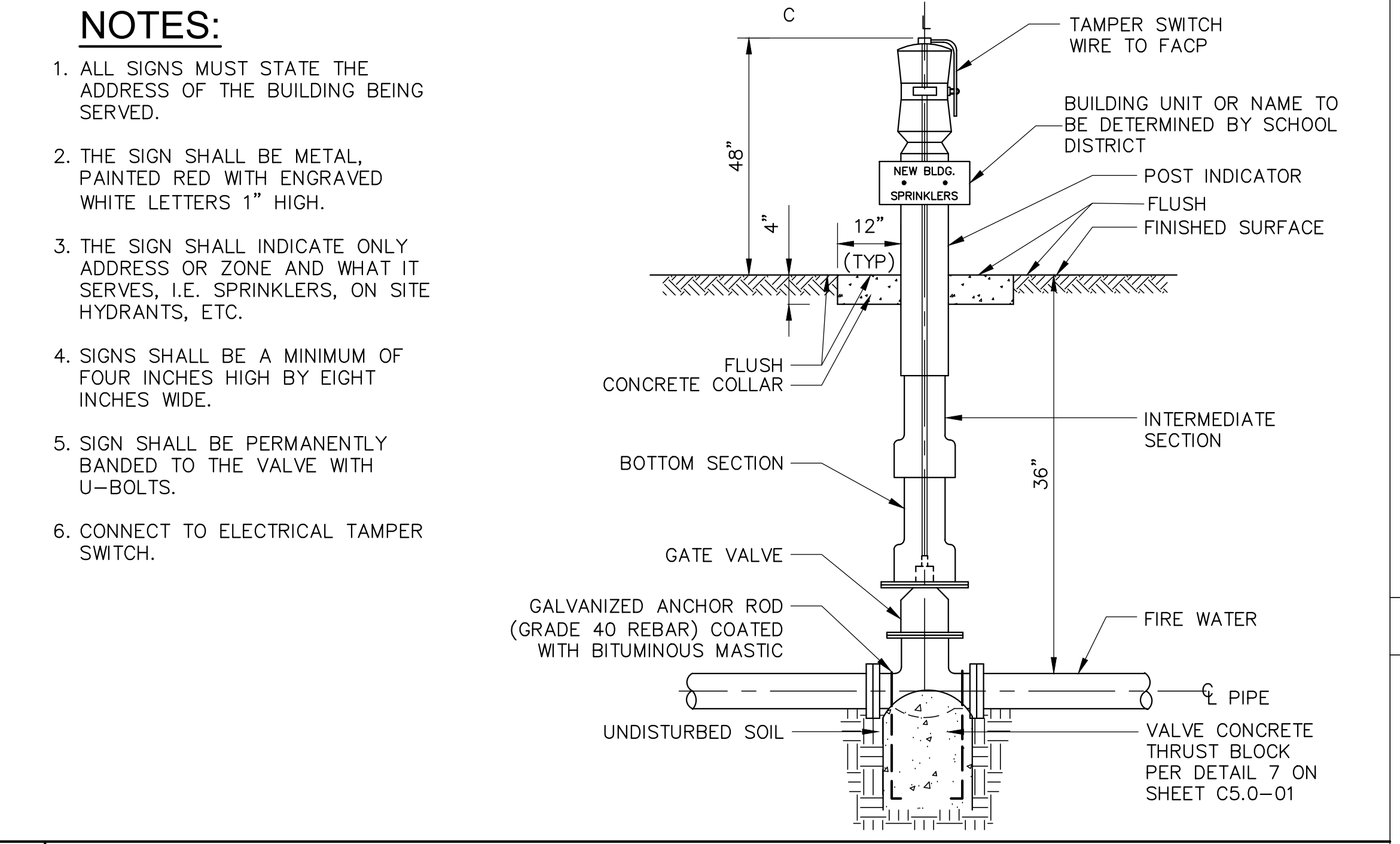
- NOTES:**
- BARRICADES, FENCES, WALLS, LANDSCAPING, ETC. SHALL NOT BE INSTALLED OR PLANTED WITHIN 3' OF A HYDRANT.
 - FIRE HYDRANT SHALL BE ONE OF THE FOLLOWING
A. CLOW / RICH - NO. 550, 555 OR 850.
B. JAMES JONES - NO. J3700 - FLUTED BARREL
C. MUELLER - A480 - E
 - HYDRANT SHALL BE SUPPLIED WITH 2-1/2" x 4" OUTLETS AND 1-1/4" OR 1-3/4" PENTAGON NUTS ON CUPS AND OPERATING VALVES.
 - HYDRANT SHALL BE PAINTED WITH O.S.H.A. SAFETY YELLOW AMERTONE 719 OR APPROVED EQUAL.
 - HYDRANT BURY, VALVE AND TEE SHALL HAVE EITHER RING-TITE JOINTS OR MECHANICAL JOINTS COMPATIBLE WITH PIPE MATERIAL USED.
 - ALL PIPE AND FITTINGS FOR HYDRANT INSTALLATION SHALL BE CLASS 200.

2 FIRE HYDRANT DETAIL
NOT TO SCALE

- NOTES:**
- PAVEMENT FINISH SURFACE SHALL BE A SMOOTH CONTINUATION OF ADJOINING PAVED SURFACE.
 - PIPELINE BEDDING MATERIAL, TRENCH BACKFILL MATERIAL, AND COMPACTION SHALL COMPLY WITH SSPWC.
 - BEDDING MATERIALS CONSISTING OF SAND, GRAVEL, OR CMB SHOULD BE USED TO BACKFILL AROUND UTILITY PIPES TO APPROXIMATELY ONE FOOT ABOVE THE TOP OF THE PIPE. ON-SITE SOILS WHICH HAVE A SAND EQUIVALENT (SE) OF 30 OR GREATER CAN ALSO BE USED AS BEDDING MATERIAL. NO MORE THAN 30% OF BACKFILL VOLUME SHOULD BE LARGER THAN 3/4". PRIOR TO PLACING THE PIPES, THE PIPE TRENCH SUBGRADE SHOULD BE OBSERVED BY A REPRESENTATIVE OF THE PROJECT GEOTECHNICAL ENGINEER. IN THE LARGEST DIMENSION, IMPORTED BACKFILL SHOULD BE APPROVED BY PROJECT GEOTECHNICAL CONSULTANT PRIOR TO DELIVERY AT THE SITE.
 - IT IS RECOMMENDED THAT UTILITY TRENCHES ARE NOT BE OR PLACED PARALLEL TO AND BELOW A 1:1 PLANE PROJECTED DOWN FROM THE BASE OF THE OUTER EDGE OF A CONVENTIONAL FOUNDATION.
 - IF THE EXPOSED SUBGRADE IS LOOSE OR UNSTABLE, THE UNSUITABLE SUBGRADE SOIL MUST BE EXCAVATED AND REPLACED WITH BEDDING MATERIAL. BEDDING MUST BE PLACED UNIFORMLY ON EACH SIDE OF THE PIPE AND MECHANICALLY COMPACTION.
 - FLOODING OR JETTING TO DENSIFY THE BEDDING MATERIALS IS NOT ALLOWED DUE TO THE CLAYEY NATURE OF ON-SITE SOILS.
 - THE BACKFILL FOR THE REMAINING PORTION OF THE TRENCH ABOVE THE PIPES SHOULD BE PLACED IN LOOSE LIFTS NOT TO EXCEED 8 INCHES, MOISTURE-CONDITIONED WITHIN OPTIMUM AND 2 PERCENT ABOVE OPTIMUM MOISTURE CONTENT, AND MECHANICALLY COMPACTION TO AT LEAST 90 PERCENT RELATIVE COMPACTION IN ACCORDANCE WITH ASTM D1557. THINNER LIFTS MAY BE NECESSARY TO ACHIEVE THE RECOMMENDED LEVEL OF COMPACTION OF THE BACKFILL DUE TO EQUIPMENT LIMITATIONS.
 - THE HIGHER COMPACTION IS REQUIRED FOR FILL MATERIAL THAT HAS LESS THAN FIFTEEN PERCENT (15%) OF THE MATERIAL FINER THAN 0.005MM.
 - TRENCHES IN PAVEMENT AREAS SHOULD BE CAPPED WITH AT LEAST 12 INCHES OF COMPACTION, ON-SITE SOIL SIMILAR TO THAT OF THE ADJOINING SUBGRADE. THE UPPER 12 INCHES OF TRENCH BACKFILL IN AREAS TO BE PAVED SHOULD BE COMPACTED TO AT LEAST 95 PERCENT RELATIVE COMPACTION. SPECIAL CARE SHOULD BE TAKEN IN THE CONTROL OF UTILITY TRENCH BACKFILLING IN THE PAVEMENT AREAS.
 - PIPELINE BEDDING MAY BE LEAN CONCRETE CONSISTING OF TWO SACKS OF PORTLAND CEMENT PER CUBIC YARD OF SLURRY IN LIEU OF SAND AS LONG AS SLURRY IS VIBRATED IN PLACE.
 - MINIMUM COVERAGE OF UTILITIES IS 36-INCHES. IF THIS CANNOT BE ATTAINED, CAP WITH 1-SACK CONCRETE SLURRY. IN PAVING AREAS, BACKFILL TRENCHES WITH SLURRY UP TO BOTTOM OF PAVING. IN LANDSCAPE AREAS, SLURRY IS ALLOWED UP TO TWO-FEET BELOW GRADE.
 - PROVIDE METALLIC WARNING TAPE 12-INCHES BELOW GRADE ABOVE UTILITIES.
 - A MINIMUM OF 6-INCH THICK BEDDING MATERIAL SHALL BE PLACED BELOW THE BOTTOM OF UTILITY LINES, ON A FIRM AND UNYIELDING SUBGRADE. THE BEDDING MATERIAL SHALL MEET THE SPECIFICATIONS PROVIDED IN THE LATEST EDITION OF THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION (GREENBOOK). SAND OR GRAVEL SHALL BE COMPACTIONED IN ACCORDANCE WITH GREENBOOK SPECIFICATIONS.
 - THE PIPE INVERT SHALL BE UNDERLAIN WITH AT LEAST 6" OF BEDDING MATERIAL CONSISTING OF SELECT SANDY SOILS WITH A SAND EQUIVALENT (SE) OF 30 OR GREATER AS SPECIFIED IN SECTION 306-1.2.1 OF THE GREENBOOK. BEDDING MATERIAL BELOW THE PIPE INVERT SHALL BE COMPACTIONED TO AT LEAST 90 PERCENT RELATIVE COMPACTION AS DETERMINED BY ASTM D1557.
 - THE PIPE ZONE BACKFILL SHALL EXTEND FROM THE INVERT OF THE PIPE TO A LEAST 6" ABOVE THE PIPE. THIS ZONE SHALL ALSO BE BACKFILLED WITH SANDY MATERIAL SIMILAR TO THE BEDDING AND MECHANICALLY COMPACTION TO 90 PERCENT RELATIVE COMPACTION.
 - BURRIED METAL PIPES SHALL BE WRAPPED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.



6 STORM DRAIN AND SEWER CLEANOUTS
NOT TO SCALE



3 POST INDICATOR VALVE DETAIL
NOT TO SCALE

- NOTES:**
- PROVIDE 8" MIN DIAMETER FOR STENCIL.
 - STENCIL IN BLUE PAINT NEAR ALL CATCH BASIN DRAINS TO READ "NO DUMPING, DRAINS TO OCEAN".
 - STENCILS MAY BE PURCHASED AT THE LOCAL COUNTY BUILDING AND SAFETY OFFICE AT 626-458-6390.

4 NO DUMPING SYMBOL
NOT TO SCALE

TABLE 1

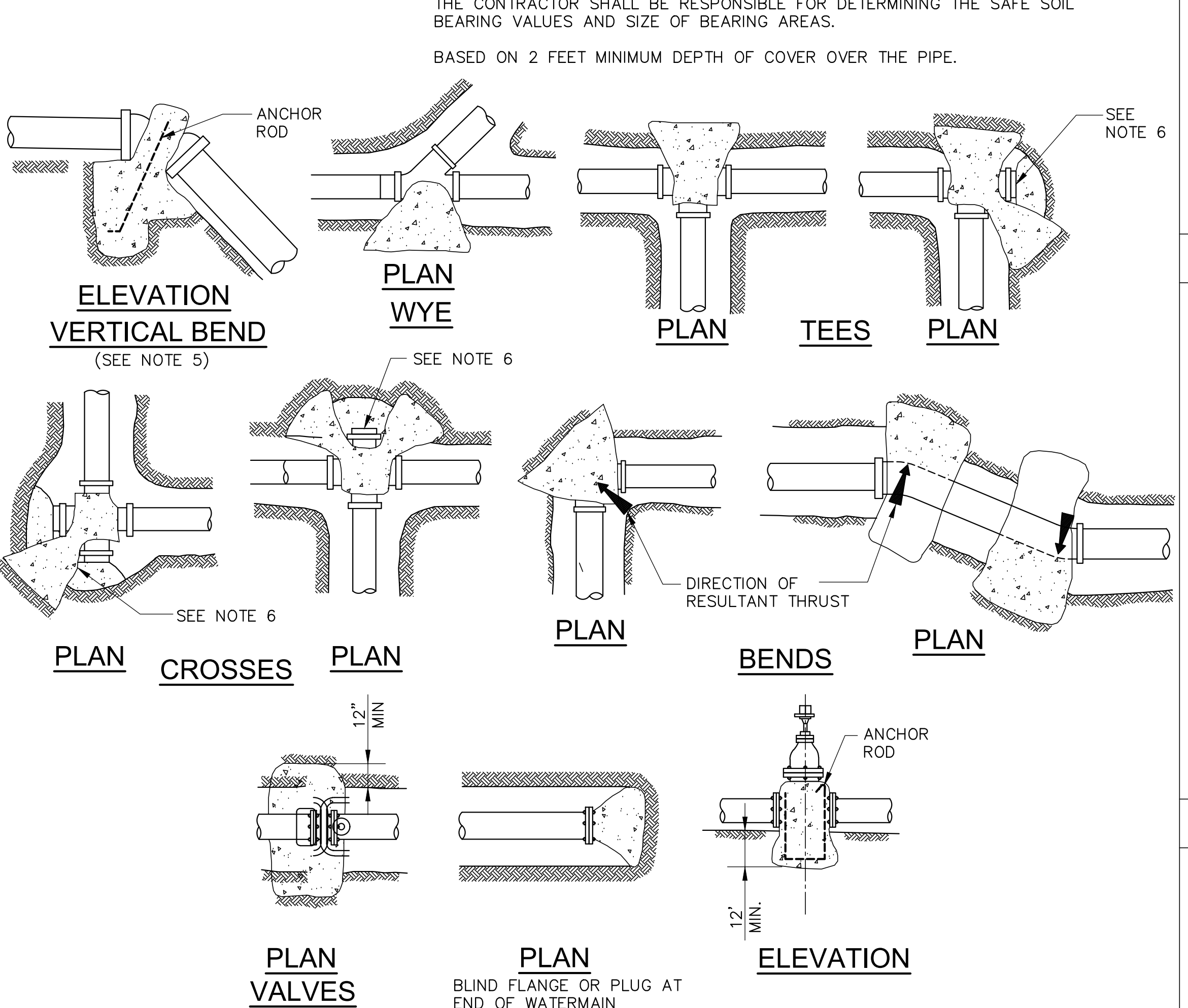
| MINIMUM BEARING AREAS IN SQ.FT. | | | | | |
|---------------------------------|-----|----------|----------|--------------|--|
| MAIN SIZE | TEE | 90° BEND | 45° BEND | 22 1/2° BEND | |
| 6" | 4 | 4 | 4 | 3 | |

BASED ON 150 PSI W.W.R. PRESSURE & SOIL BEARING LOADS OF 2000 PSF THE RATIO OF WIDTH TO HEIGHT SHALL NOT EXCEED 1 1/2 TO 1
TEES, PLUGS, CAPS & HYDRANTS.

TABLE II

| SOIL TYPE | MAX. ALLOWABLE SOIL BEARING VALUES | FACTORS FOR INCREASING AREAS IN TABLE 1 |
|---------------------|------------------------------------|---|
| LOOSE SAND | 500 PSF | 4 |
| SOFT SANDY CLAY | 1000 PSF | 2 |
| ADOBE | 1000 PSF | 2 |
| COMPACT FINE SAND | 2000 PSF | 1 |
| COMPACT COARSE SAND | 2000 PSF | 1 |
| MEDIUM STIFF CLAY | 2000 PSF | 1 |

THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE SAFE SOIL BEARING VALUES AND SIZE OF BEARING AREAS.
BASED ON 2 FEET MINIMUM DEPTH OF COVER OVER THE PIPE.



- GENERAL NOTES:**
- ALL ANCHOR AND THRUST BLOCKS SHALL BEAR AGAINST UNDISTURBED SOIL.
 - MINIMUM ALLOWABLE WATER PRESSURE FOR DESIGN OF THRUST BLOCKS IS 150 PSI. BEARING AREA INCREASE IN PRESSURE.
 - ALL CONCRETE USED IN THRUST BLOCKS SHALL ATTAIN 2000 PSI STRENGTH.
 - ALL ANCHOR RODS SHALL BE REINFORCING STEEL AND A MINIMUM OF 1/2-INCH IN DIAMETER.
 - USE ANCHOR BLOCKS AT VERTICAL BENDS WHEN PIPE IS ABOVE OR BELOW GROUND. SIZE OF BLOCK AND ROD SHALL BE AS SHOWN ON THE PLANS OR AS DETERMINED BY THE ENGINEER IN THE FIELD.
 - USE 30 POUND FELT TO INSURE COLD JOINT.
 - CONCRETE SHALL NOT COME INTO DIRECT WITH ASBESTOS CEMENT PIPE.
 - FOR PIPE 14" IN DIAMETER OR LARGER ENGINEER IS TO SUBMIT CALCULATIONS.

7 THRUST BLOCK DETAILS
NOT TO SCALE

5 TRENCH SECTION
SCALE: NOT TO SCALE

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SEAL

REGISTERED ARCHITECT
AMMAR NAJI
SARBAM
C-30902
12/31/2023
IN THE
STATE OF CALIFORNIA

A# 03-123205 INC: 01

CONSULTANTS

1041 S. Garden Avenue, Suite #210
Anaheim, CA 92801
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PROJECT TITLE
**COMPTON COLLEGE
STUDENT HOUSING**
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK &
UNDERGROUND UTILITIES
1111 E. ARETASIA BLVD., COMPTON, CA 90221

ISSUED

| # | DATE | DESCRIPTION |
|---|------------|-------------|
| 1 | 03/01/2024 | REVISION A |

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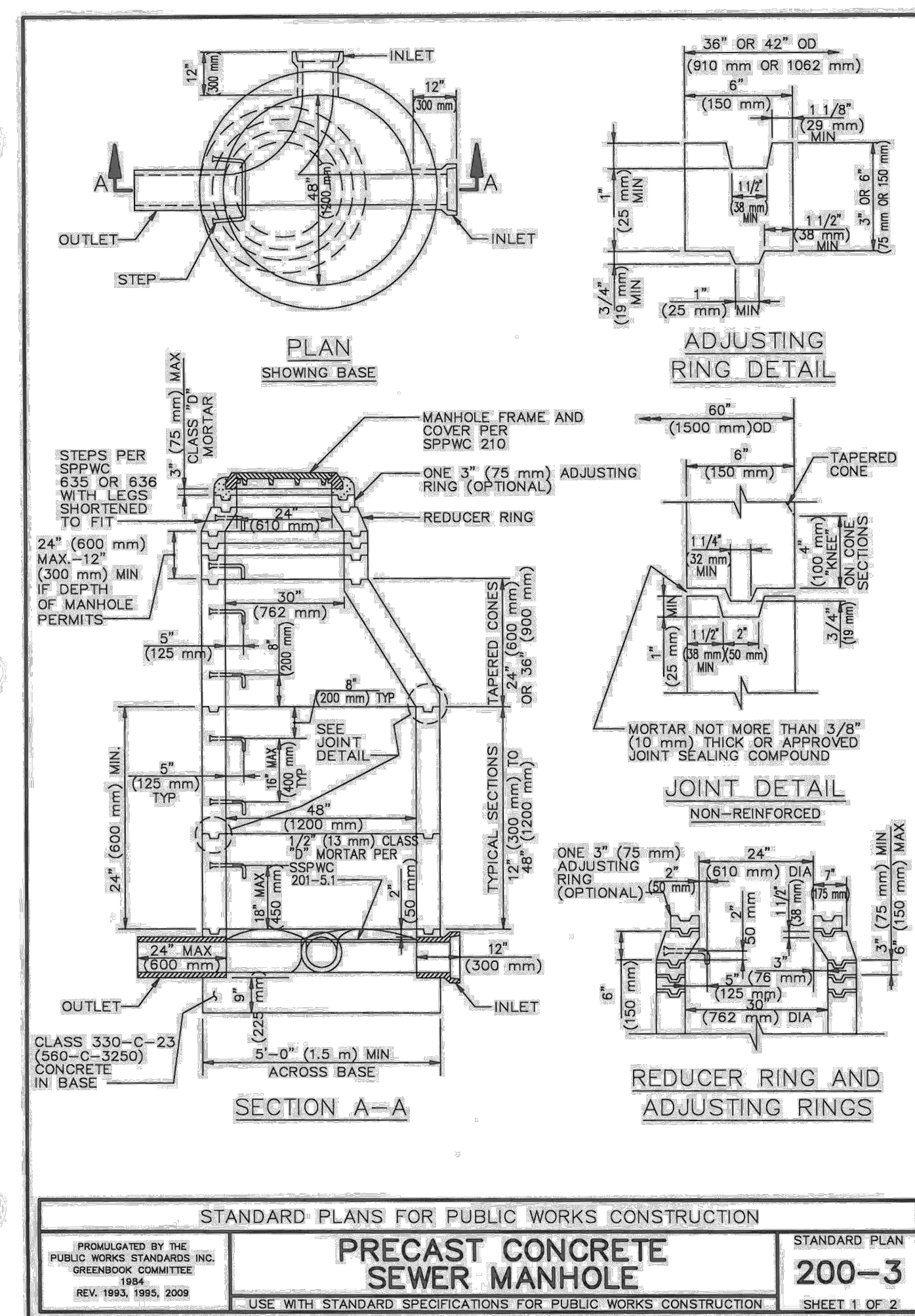
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MISCELLANEOUS
DETAILS

SHEET NUMBER

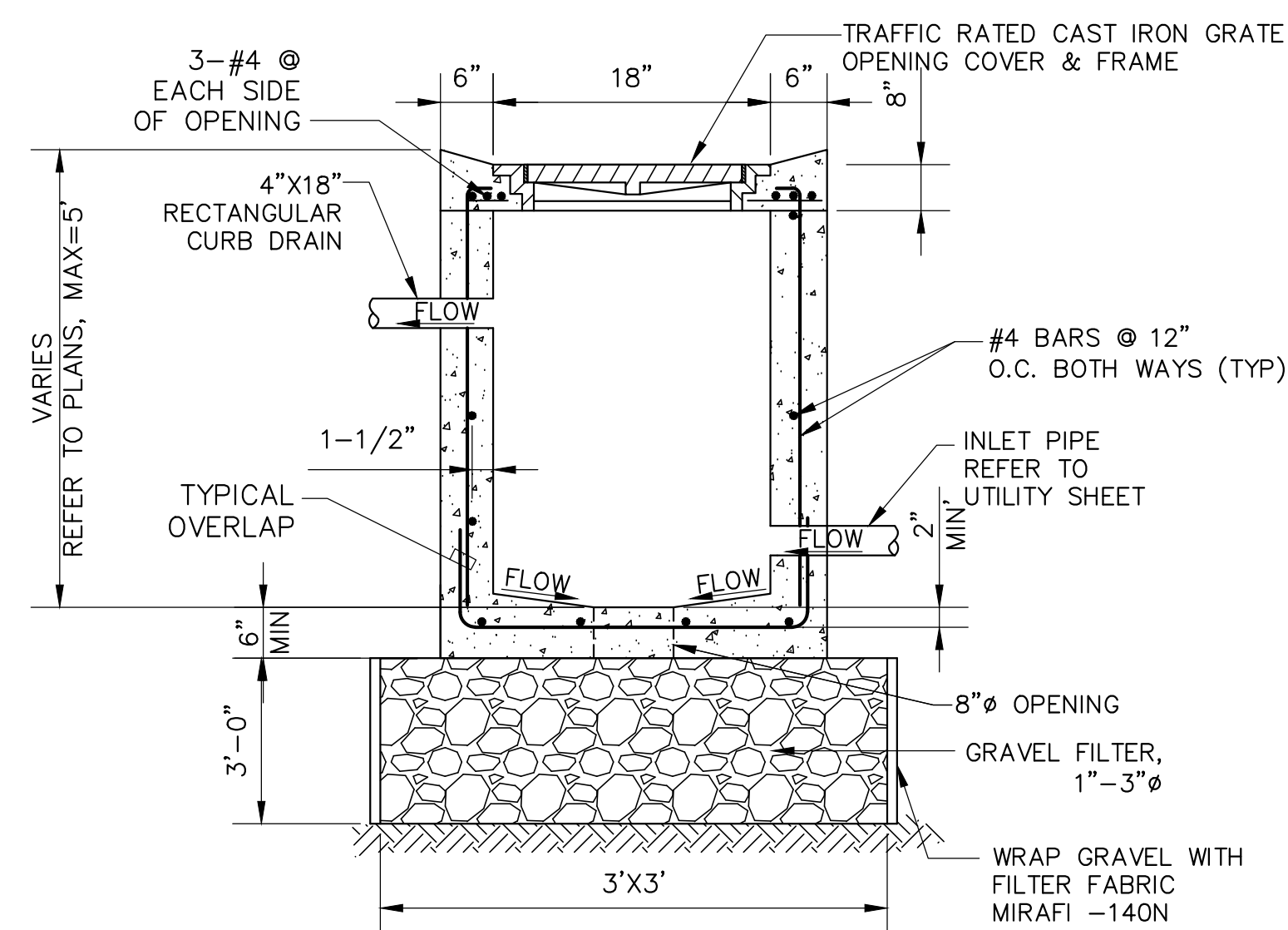
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CONSTRUCTION DOCUMENTS

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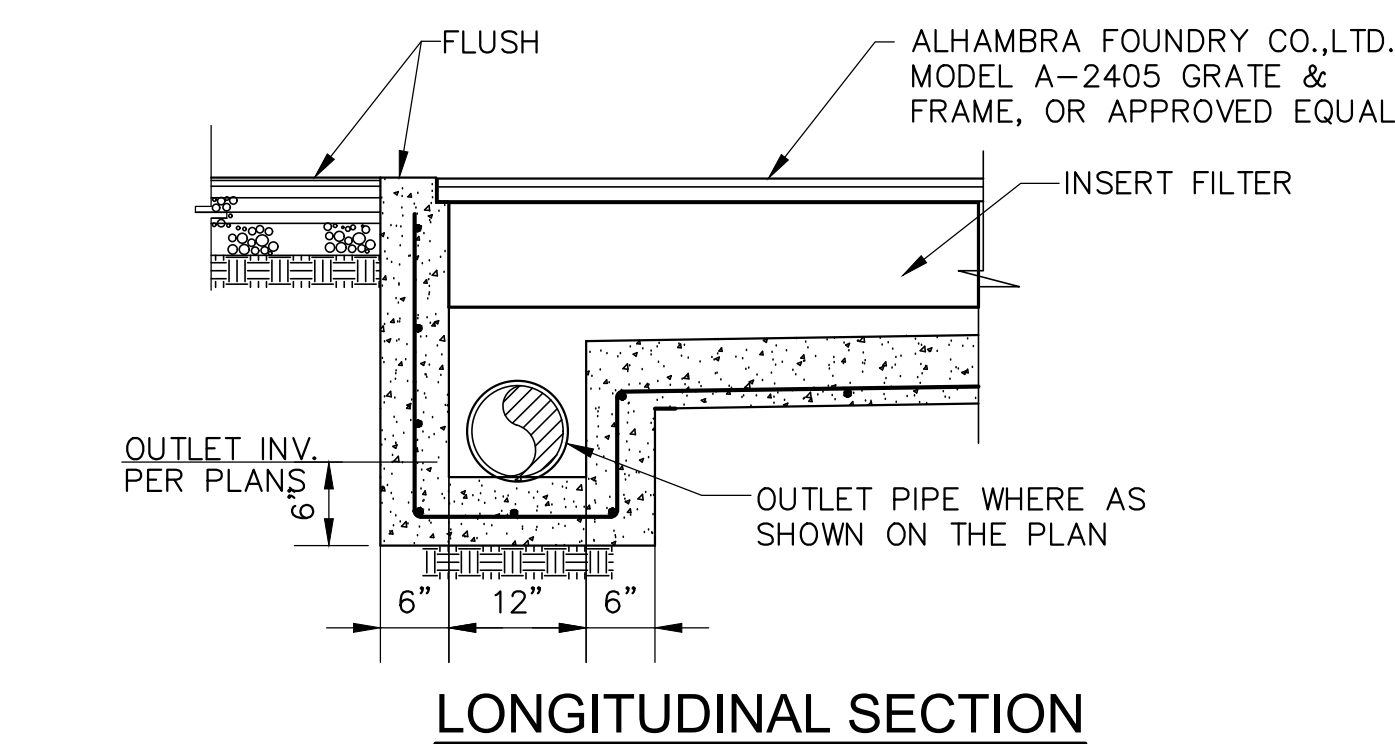
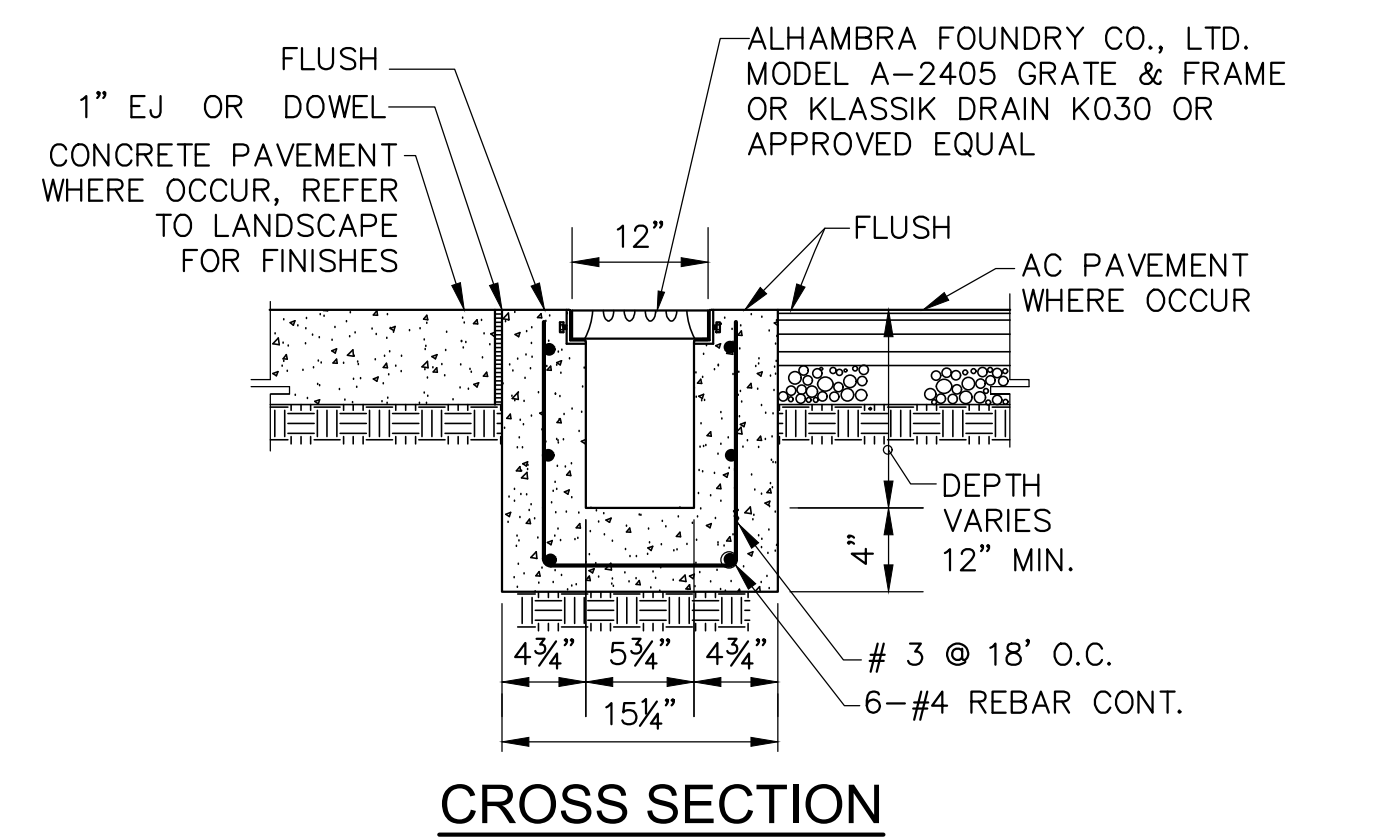
- NOTES:**
- EXCEPT AS NOTED HEREON, THE PRECAST UNITS SHALL BE MANUFACTURED AND TESTED IN ACCORDANCE WITH ASTM C 478. AS AN ALTERNATE CURING METHOD, THE UNITS MAY BE CURED USING SATURATED STEAM FOR A MINIMUM OF 12 HOURS FOLLOWED BY 5 DAYS OF WATER CURING OR MEMBRANE CURING. IF THE UNITS ARE CURED BY THE ALTERNATE METHOD, THEY SHALL NOT BE SHIPPED PRIOR TO 8 DAYS AFTER CASTING NOR UNTIL THE CONCRETE HAS ATTAINED A STRENGTH OF 3500 PSI (25 MPa).
 - MANHOLE STEPS SHALL CONFORM TO SPPWC 636 TYPE 1 OR 3 OR SPPWC 636. THE MANHOLE STEPS SHALL BE UNIFORMLY SPACED AT A MAXIMUM OF 18" (400 mm). THE LOWEST STEP SHALL BE PLACED NOT LESS THAN 8" (200 mm) NOR MORE THAN 18" (450 mm) ABOVE THE SHELF. THE STEPS SHALL PROJECT 5" (125 mm) INSIDE THE MANHOLE.
 - RISER SECTIONS MAY BE REINFORCED OR UNREINFORCED. REINFORCED SECTIONS SHALL BE REINFORCED IN ACCORDANCE WITH ASTM C 478 AND SHALL HAVE A MINIMUM WALL THICKNESS OF 5" (125 mm). UNREINFORCED RISER SECTIONS SHALL HAVE A MINIMUM WALL THICKNESS OF 6" (150 mm).
 - THE 24"x48" (600 mm x 1200 mm) ECCENTRIC CONES MAY BE REINFORCED OR UNREINFORCED. IF REINFORCED, THE WALL THICKNESS SHALL BE NOT LESS THAN 5" (125 mm). IF UNREINFORCED, THE WALL THICKNESS SHALL NOT BE LESS THAN 6" (150 mm).
 - JOINTS SHALL BE TONGUE AND GROOVE. JOINTS FOR REINFORCED STRUCTURES SHALL CONFORM WITH ASTM C 478 SECTION 14.
 - PRECAST UNITS SHALL BE ASSEMBLED USING CLASS "B" MORTAR.
 - IF 30" (762 mm) DIAMETER MANHOLE FRAME AND COVER IS REQUIRED, IT SHALL BE INSTALLED WHERE THE REDUCER RING IS SHOWN IN THE SECTION.
 - FOR REINFORCED PRECAST STRUCTURES, ALL REINFORCEMENT SHALL HAVE A MINIMUM OF 2" (50 mm) OF COVER OVER THE STEEL ON THE INSIDE FACE.
 - THE TOP OPENING OF THE MANHOLE AND THE STEPS SHALL BE PLACED DIRECTLY OVER THE OUTLET OF THE STRUCTURE EXCEPT AS OTHERWISE NOTED ON PLANS.
 - CONCRETE BASE AND STUB WALLS SHALL BE POURED IN ONE OPERATION TO A POINT 2" (50 mm) ABOVE THE INLET AND OUTLET PIPES. ALL PIPES SHALL BE RIGIDLY SUPPORTED BY TEMPORARY PIERS OR OTHER METHODS DURING THE OPERATION. CONCRETE SHALL SET FOR 24 HOURS BEFORE PLACING PRECAST UNITS.



- NOTES:**
- USE 3/4" DIA. PIPE BAR SPACERS ASSEMBLED ON (2) 1/2" DIA. RODS WITH THREADS AND NUTS AT BOTH ENDS.
 - ALL METAL PARTS SHALL BE GALVANIZED AFTER FABRICATION AND WELDING, AND BEFORE ASSEMBLING.
 - GRATES SHALL BE OF VANDAL-RESISTANT CONSTRUCTION WITH 1/2" MAX OPENINGS.
 - FRAME AND GRATE SHALL BE TRAFFIC-RATED WHEN INSTALLED IN PAVED (ASPHALT OR CONCRETE) AREAS.
 - GRATE MUST COMPLY WITH ADA REQUIREMENTS WHERE REQUIRED.
 - PROVIDE 1/2" MAX GRID/OPENINGS IN GRATING IN THE DIRECTION OF TRAFFIC FLOW.
 - PROVIDE NO DUMPING SYMBOL PER DETAIL 8 ON THIS SHEET.

1 SEWER MANHOLE DETAIL
NOT TO SCALE

2 BUBBLER CATCH BASIN DETAIL
NOT TO SCALE

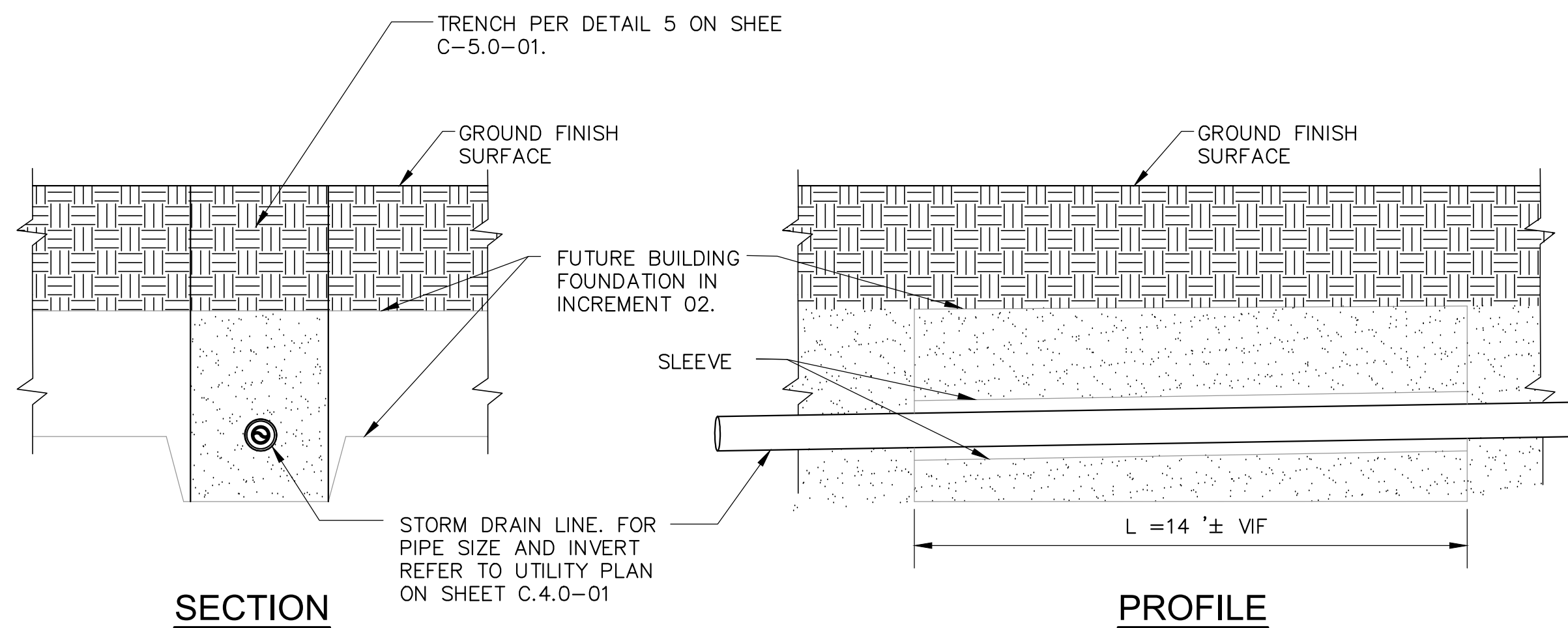


- NOTES:**
- GRATE MUST COMPLY WITH ACCESSIBILITY REQUIREMENTS, HEEL PROOF, VANDAL RESISTANT AND TRAFFIC RATED.
 - CONTRACTOR CAN ALSO USE PRE-CAST TRENCH DRAINS. SUBMIT MANUFACTURER'S CATALOG AND SHOP DRAWING (IF APPLICABLE) FOR APPROVAL.
 - CONTRACTOR SHALL COORDINATE THE CONCRETE FINISH WITH LANDSCAPE DRAWINGS.

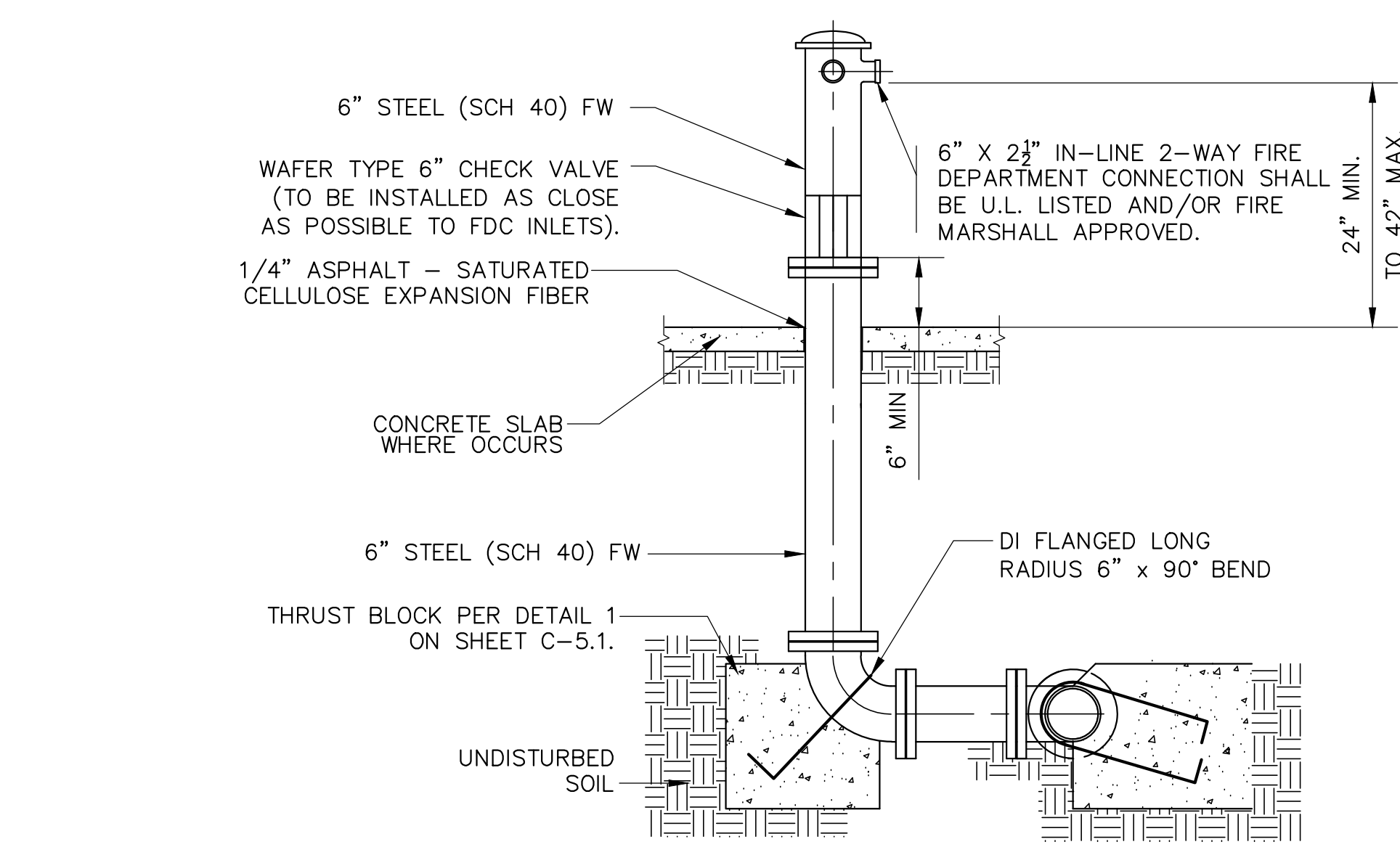
3 TRENCH DRAIN DETAIL
NOT TO SCALE



4 AREA DRAIN DETAIL
NOT TO SCALE



6 PIPE/CONDUIT PENETRATION AT FUTURE FOOTING
NOT TO SCALE

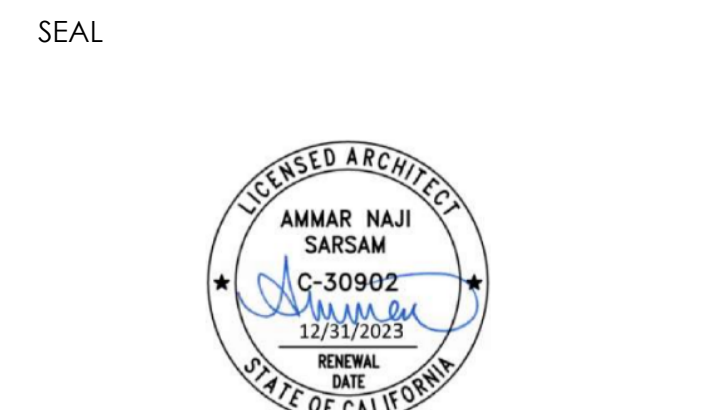


5 FIRE DEPARTMENT CONNECTION
NOT TO SCALE

DSA STAMP

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 03-123205 INC.
REVIEWED FOR:
SS [x] FLS [x] ACS [x]
DATE: 10/02/2023

HPI
architecture
www.hpiarchitecture.com
115 22nd street
Newport Beach, CA
92663
o: 949.675.6442



CONSULTANTS

MC A ENGINEERS, INC.
1041 S. GARDEN AVENUE, SUITE 9210
ANNEBORO, CA 92710
TEL: 323.728.6098 FAX: 323.728.6043



PROJECT TITLE
COMPTON COLLEGE STUDENT HOUSING
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, & UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD., COMPTON, CA 90221



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MISCELLANEOUS
DETAILS

SHEET NUMBER
C-5.1-01

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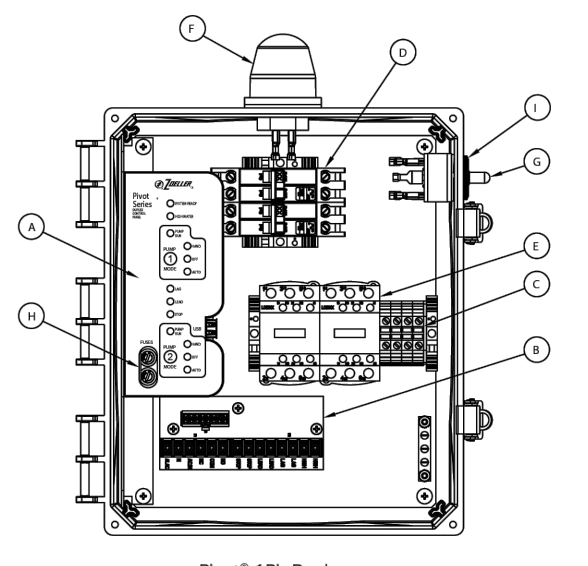
Pivot® Pro, Pivot® Pro, Pivot® Pro+ Selection Guide

Pivot® Pro, 1Ph only, Simplex and Duplex

- Standard Features: 1. NEMA 4X 12"X10"X6" enclosure with lockable hasp... 2. CCS&US labeled. Certified to UL508... 3. Input power terminal block... 4. Float switch terminal block... 5. Ground lug... 6. IEC rated motor connectors... 7. Control circuit powered by 115vac, 60 hz... 8. Alarm circuit can be powered separately from circuit breaker... 9. Control and alarm fuses... 10. External red globe for fuses... 11. Selectable Audible 95 decibel (at 2' (0.6 m) horn can be set to either auto or latching (Manual Alarm Reset)... 12. External alarm test and silence switch... 13. User interface buttons and leds... 14. Float status indicator LED indicators... 15. Float smart logic for pump run priority... 16. Pump Run LED indicators... 17. Non-simultaneous pump start... 18. Pump-run dry contact, normally open... 19. Form C auxiliary output dry contact for alarm conditions... 20. Smart HAND-OFF-AUTO (H/OA)... 21. Alarms: High voltage, Disabled alarm circuit, Float fault, Continuous pump run, Service off timerout, Failed contactor... 22. USB access to: Elapsed Time Meter, Event Counter

- 23. Configurable settings: Simplex or duplex mode, 3 or 4 floats, Float order SLH, SLHL, Smart or relay logic, Alarm based, solid or blinking globe, Active or latching alarm, Continuous run timer, H/OA hand timer, H/OA service off alarm timer... 24. Limited warranty: 5 years

*Pivot® panels are only offered with standard features above. For additional features, upgrade to Pivot® Pro. For any other options not shown, please refer to Pivot (R) Pro Series.



Pivot® Pro 1Ph Duplex

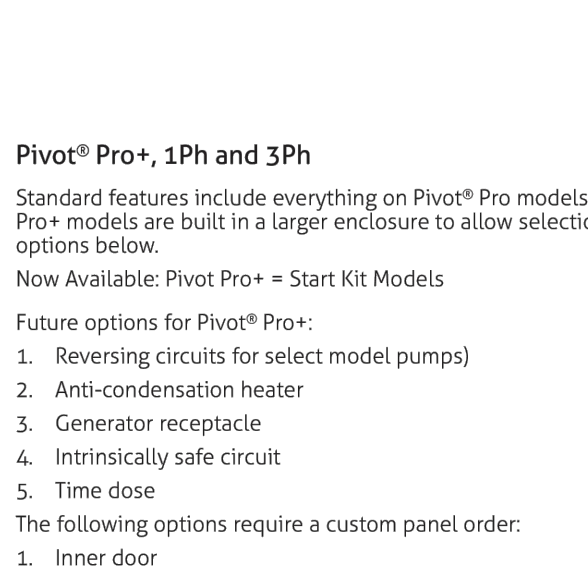
Pivot® Pro, 1Ph and 3Ph includes all Standard PIVOT(R) features shown above with these additional features: 1. Multi-tap transformer (3ph) 2. Pump circuit breaker (1Ph and 3Ph) or overload (3Ph) 3. User interface with LCD screen/menu, buttons, LEDs, D seal, fail adjustment screw 4. Seal fail indicator and adjustment, horn/globe/display latching 5. Thermal cutoff indicator, horn/globe/display latching 6. Dry Contact auxiliary includes Seal/Thermal conditions 7. Alternator 1-2 selector switch (lead/lag selector switch) 8. Z Control* enabled for remote monitoring/control and alert notifications (purchase of gateway required) 9. Additional alarms: Seal fail (moisture) alarm, adjustable sensitivity, Thermal indicator trip alarm, Tripped overload (3ph only)

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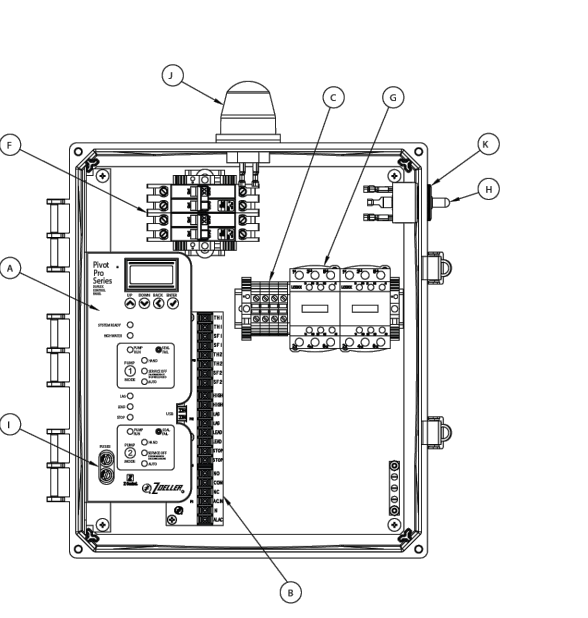
Pivot® Pro+, 1Ph and 3Ph

- Standard Features: 1. NEMA 4X 12"X10"X6" enclosure with lockable hasp... 2. CCS&US labeled. Certified to UL508... 3. Input power terminal block... 4. Float switch terminal block... 5. Ground lug... 6. IEC rated motor connectors... 7. Control circuit powered by 115vac, 60 hz... 8. Alarm circuit can be powered separately from circuit breaker... 9. Control and alarm fuses... 10. External red globe for fuses... 11. Selectable Audible 95 decibel (at 2' (0.6 m) horn can be set to either auto or latching (Manual Alarm Reset)... 12. External alarm test and silence switch... 13. User interface buttons and leds... 14. Float status indicator LED indicators... 15. Float smart logic for pump run priority... 16. Pump Run LED indicators... 17. Non-simultaneous pump start... 18. Pump-run dry contact, normally open... 19. Form C auxiliary output dry contact for alarm conditions... 20. Smart HAND-OFF-AUTO (H/OA)... 21. Alarms: High voltage, Disabled alarm circuit, Float fault, Continuous pump run, Service off timerout, Failed contactor... 22. USB access to: Elapsed Time Meter, Event Counter

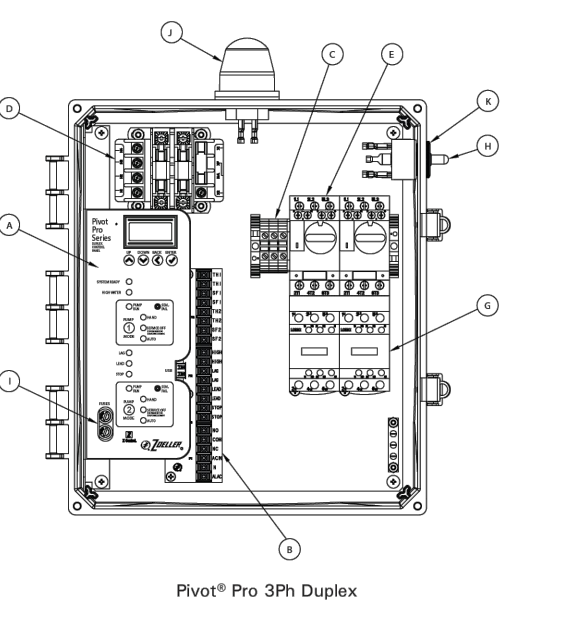
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Pivot® Pro+ 1Ph Duplex



Pivot® Pro 1Ph Duplex



Pivot® Pro 3Ph Duplex

Standard Solids Handling Pumps 62 HD Series, 5-20 BHP. Table with columns: Model, Voltage, Phase, Tag Amps, Amp Range, Simplex Panels, Duplex Panels.

Standard Solids Handling Pumps 64 HD Series. Table with columns: Model, Voltage, Phase, Tag Amps, Amp Range, Simplex Panels, Duplex Panels.

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SECTION: 2.50.090

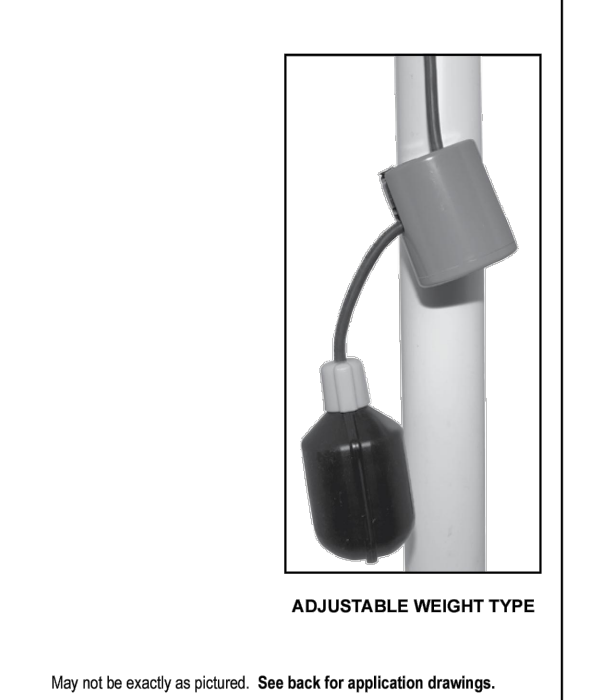
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Visit our website: zoellerpumps.com

COMPARE THESE FEATURES

- Float is constructed of durable PVC/polypolyethylene encasing variable level switch... Standard mechanical variable level control switches are rated for 115/230V, 5 Amps... Low current mechanical variable level control switches are rated for 125/250VDC, 0.1 Amps... 18/2 Type 316 S/SWJ CORD standard... Temperature rating of 140°F (60°C)... Approximately 1/2" liquid level differential in switching action... Variable Level Control Switch is normally open when hanging vertically above liquid level. Switch closes when it reaches a few degrees above the horizontal position.

VARIABLE LEVEL CONTROL SWITCHES

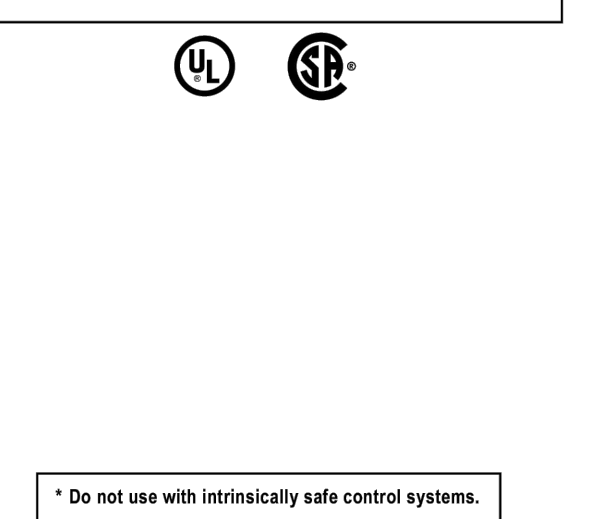


ADJUSTABLE WEIGHT TYPE

- Switch for simplex or duplex pump control and high level alarm on electrical alternating control panel systems for dewatering, effluent and sewage applications... Switch for APAct™ (use low current models)... High level alarm switch

ADJUSTABLE WEIGHT: (PIN 10-0889) provides an accurate pivot point for suspended float switches... Clipper teeth on clip and weight channel securely lock float cable into place... Cable weight can be adjusted without the use of tools... Impregnated & non-combustible, PVC housing for liquids up to 140° F (60° C)... CLIP: Hyper-rod model speed packing... WIREABLE ACCOMMODATED: S/SWJ S/UWJ: 162, 180, 183, 142, 143 BHP WEIGHT: 1.5 to 102 LB

Table with columns: Mechanical, Low Current, Cord Length, Mounting Method. Lists various float switch models and their specifications.



Do not use with intrinsically safe control systems. For use with intrinsically safe control systems only.

Your Peace of Mind is Our Top Priority™



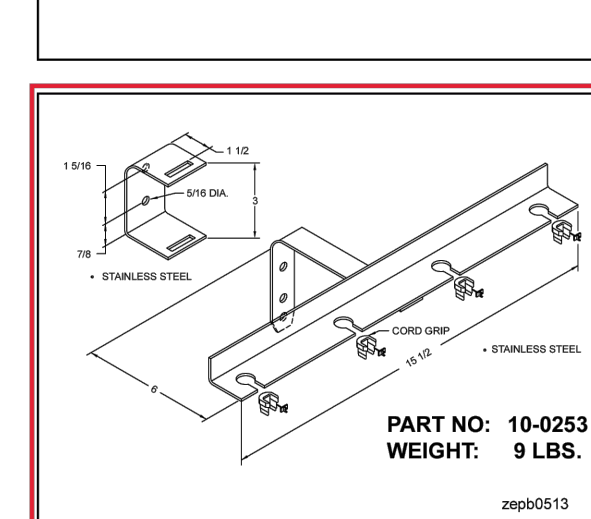
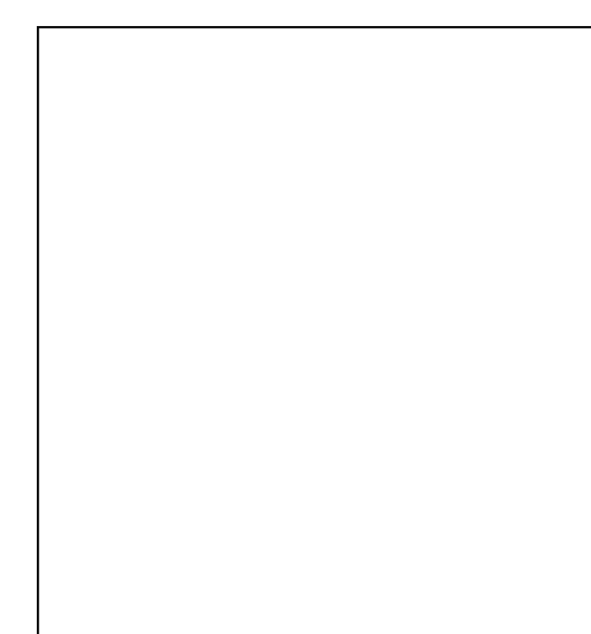
SECTION: 2.50.110

MAIL TO: P.O. BOX 16547 Louisville, KY 40216-0547

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Float Switch Brackets

Also see FM526 for Float Tree Assemblies for floats that use clamp type hangers.



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A# 03-123205 INC: 01

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2"- 8" FLANGED CHECK VALVES

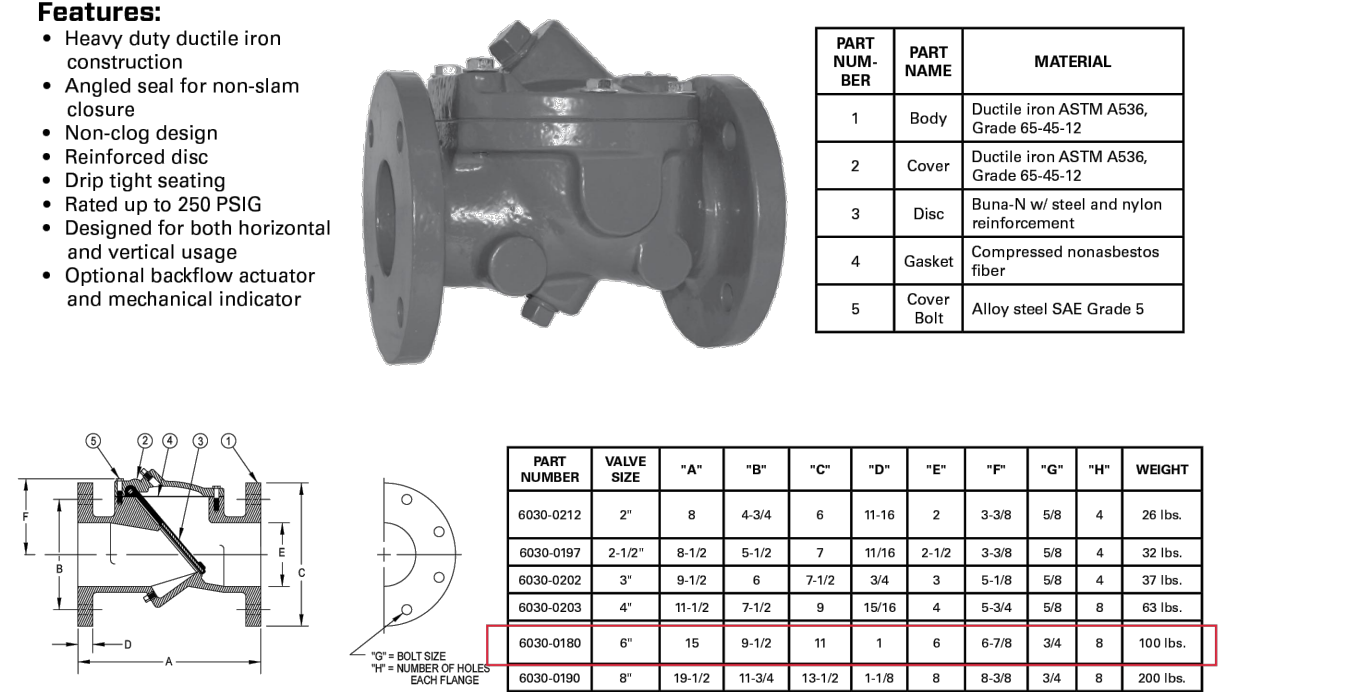


Table with columns: Part Number, Model Size, Material, Weight. Lists specifications for various check valve models.

FLANGED RAIL SYSTEMS (2-1/2" TO 6" Discharge) FIELD ASSEMBLED

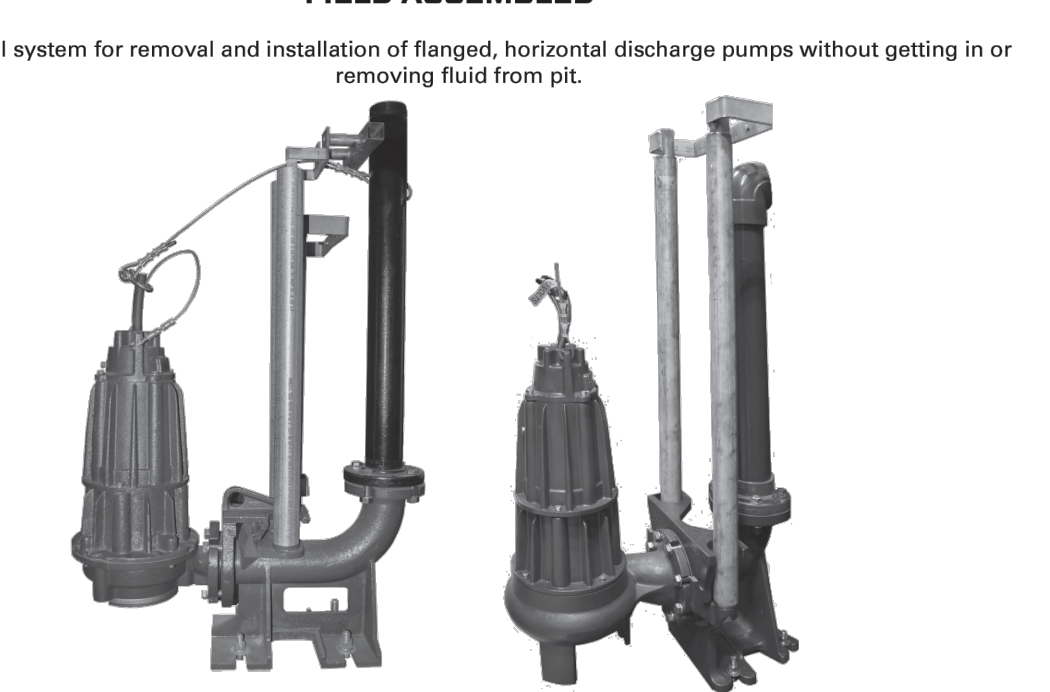


Table with columns: Part Number, Description, Pump Discharge, Rail System, Guide Rail. Lists specifications for rail system components.

PUMP LIFTING CABLES

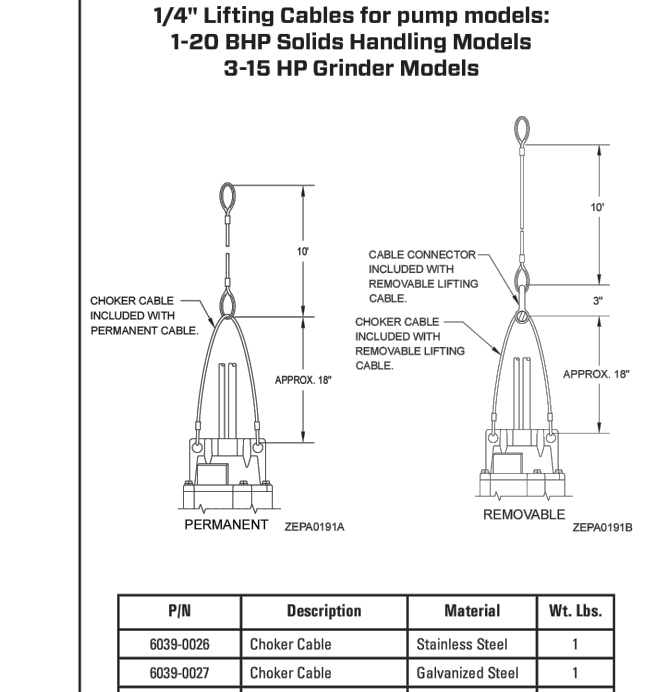


Table with columns: Part Number, Description, Material, Wt. Lbs. Lists specifications for various lifting cable models.

Rigid Lifting Ball

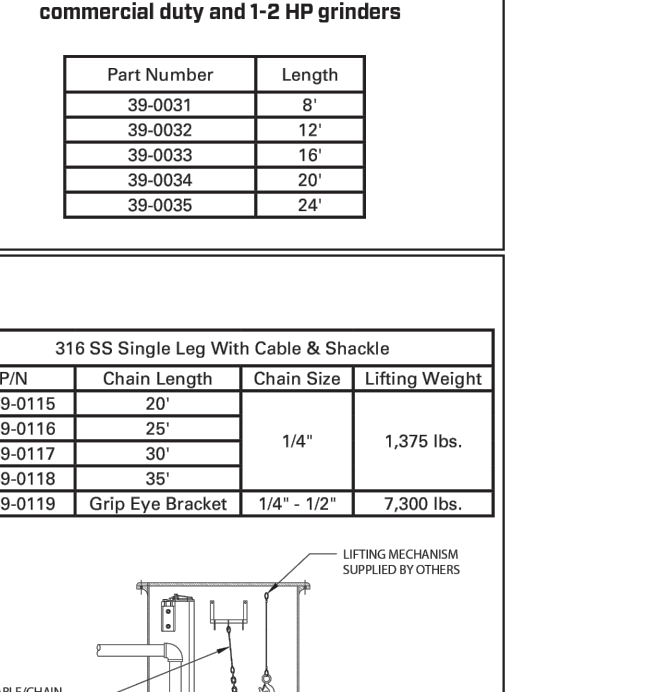


Table with columns: Part Number, Description, Material, Wt. Lbs. Lists specifications for lifting ball models.

FIBERGLASS BASIN SPEC SHEET

Technical drawing of a fiberglass basin with dimensions and specifications. Includes a table for reinforcement materials and a table for pump mounting studs.

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REGISTERED ARCHITECT
AMMAR NAJI SARHAM
 C-30902
 CIVIL
 12/17/2023
 STATE OF CALIFORNIA

CONSULTANTS
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 ANAHEIM, CA 92805
 TEL 714 729 8098 FAX 714 729 6043

REGISTERED PROFESSIONAL ENGINEER
ELIOT CHEN
 CIVIL
 No. 62622
 Exp. 8/20/24
 STATE OF CALIFORNIA

PROJECT TITLE
**COMPTON COLLEGE
 STUDENT HOUSING**
**INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
 UNDERGROUND UTILITIES**
 1111 E. ARTESIA BLVD., COMPTON, CA 90221



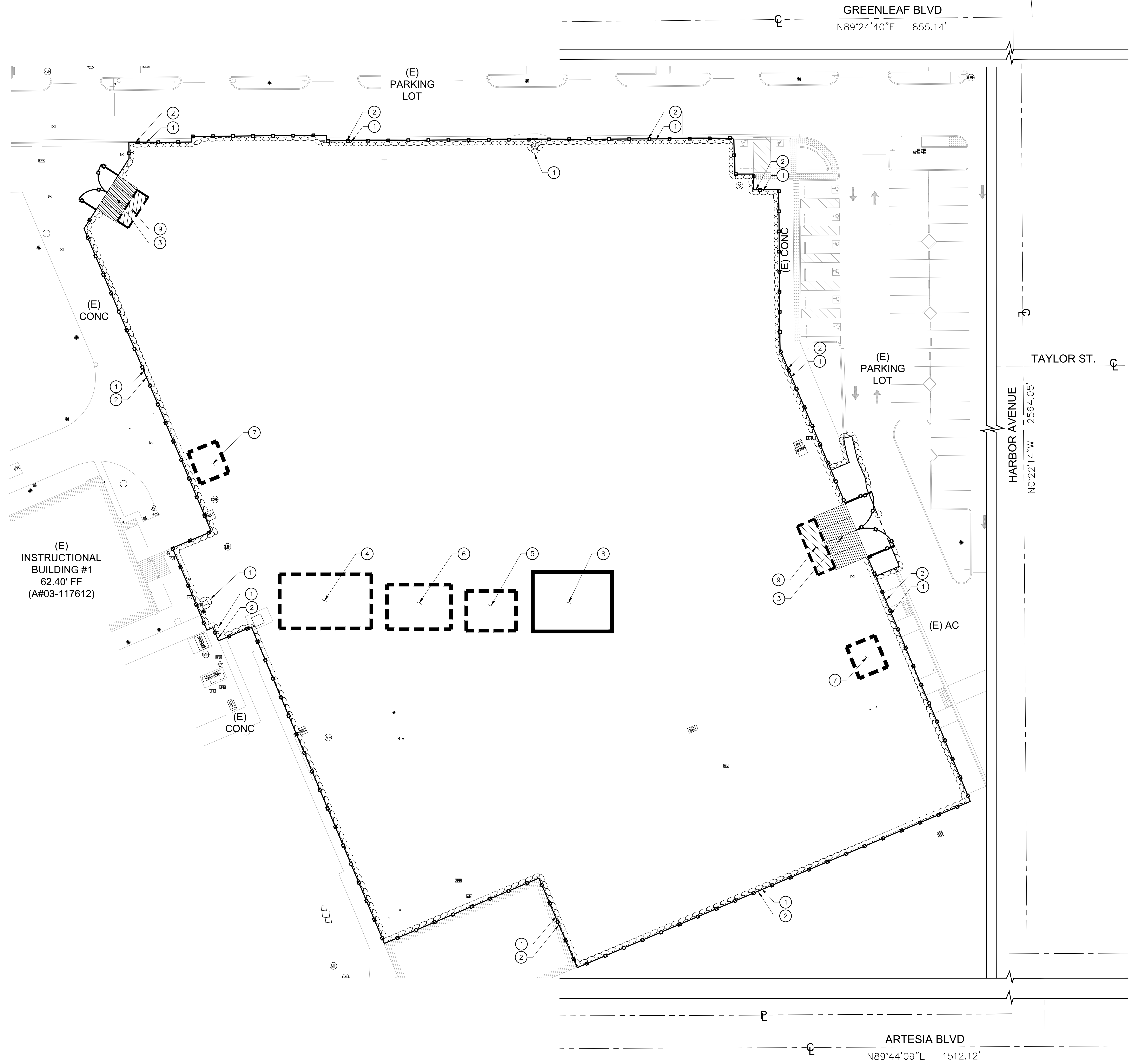
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SHEET TITLE
EROSION CONTROL PLAN

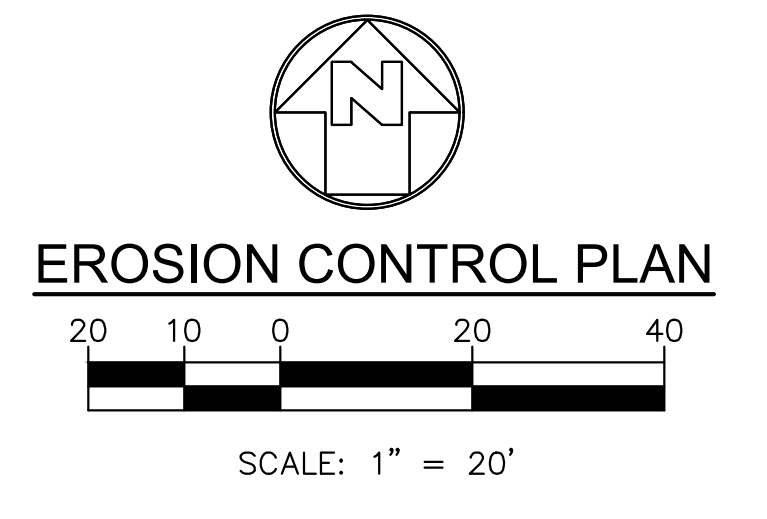
SHEET NUMBER
C-6.0-01

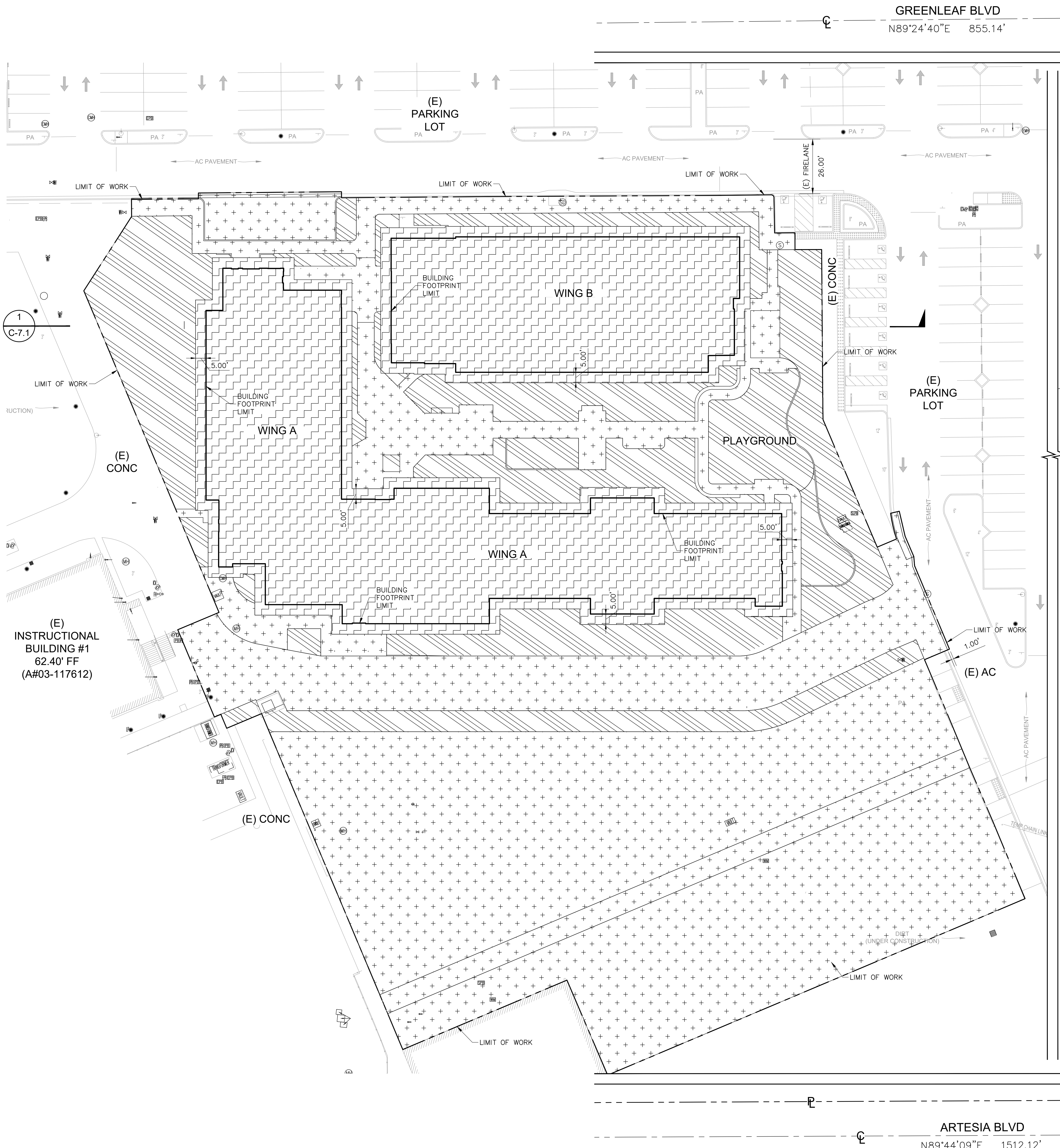


- CONSTRUCTION NOTES:**
1. INSTALL GRAVEL BAGS AND MAINTAIN THROUGHOUT THE ENTIRETY OF THE PROJECT. REFER TO DETAIL 2 ON SHEET C-6.1-01.
 2. CONSTRUCTION FENCE PER DETAIL 4 ON SHEET C-6.1-01.
 3. STABILIZED CONSTRUCTION ENTRANCE/EXIT PER DETAIL 9 AND DETAIL 3 ON SHEET C-6.1-01.
 4. PROPOSED AREA FOR EQUIPMENT STAGING. CONTRACTOR TO VERIFY EXACT LOCATION AND COORDINATE WITH THE CPM.
 5. PROPOSED AREA FOR FUELING/OILING. CONTRACTOR TO VERIFY ACTUAL AREA NEEDED AND COORDINATE WITH THE CPM. REFER TO DETAIL 8 ON SHEET C-6.1-01.
 6. PROPOSED AREA FOR LOADING. CONTRACTOR TO VERIFY EXACT LOCATION AND COORDINATE WITH THE CPM.
 7. PROPOSED AREA FOR TEMPORARY TOILETS. CONTRACTOR TO VERIFY EXACT LOCATION AND COORDINATE WITH THE CPM.
 8. PROPOSED AREA FOR VEHICLE AND EQUIPMENT CLEANING. CONTRACTOR TO VERIFY EXACT LOCATION AND COORDINATE WITH THE CPM. REFER TO DETAIL 5 ON SHEET C-6.1-01.
 9. TIRE WASH PER DETAIL 7 ON SHEET C-6.1-01.

- LEGEND:**
- GRAVEL BAG
 - CONSTRUCTION PERIMETER 8' HIGH FENCE AND GATE WITH VISUAL BARRIER

- SHEET NOTES:**
1. LOCATION FOR ANY DESIGNATED STOCKPILES SHALL BE COORDINATED AND DETERMINED BY THE CONTRACTOR ON-SITE. CONTRACTOR SHALL APPLY ALL APPLICABLE BMP'S TO PROTECT THE STOCKPILE AS OUTLINED IN DETAIL 6 ON SHEET C-6.1-01.
 2. INSTALL 2" OF TEMPORARY GRAVEL ON ALL ON-SITE CONSTRUCTION ROADWAYS TO STABILIZED AND CONTROL EROSION.
 3. CONTRACTOR SHALL MONITOR THE CONSTRUCTION SITE TO CLEAN AND SWEEP MATERIALS TRACKED OFF SITE.
 4. ALL BMP'S, SILT FENCES, ETC., SHALL BE MONITORED AND MAINTAINED BY THE NTP1 CONTRACTOR FOR THE ENTIRE DURATION OF THE CONTRACT.
 5. CONTRACTOR SHALL MONITOR WASTEWATER DISCHARGE (INCLUDING STORM RUN OFF) TO ENSURE IT MEETS STANDARDS SET BY APPROPRIATE LAWS, CODES, REGULATIONS, ORDINANCES AND PERMITS. PROVIDE A SETTLING BASIN AND OIL SEPARATOR PRIOR TO ITS DISCHARGE TO CITY OR COUNTRY SEWERS. PROVIDE A WATER SAMPLING STATION DOWNSTREAM OF BASIN FOR MONITORING OF WASTE WATER. DISPOSE OF WASTEWATER IN CLOSED CONDUITS SO AS NOT TO DAMAGE PUBLIC OR PRIVATE PROPERTY NOR CREATE A NUISANCE OR HEALTH HAZARD.
 6. CONTRACTOR SHALL NOT DISCHARGE POLLUTANTS DOWNSTREAM OF THE SETTLING BASIN/OIL SEPARATOR. THESE POLLUTANTS INCLUDE LUBRICANTS, FUELS, CHEMICALS, AND BITUMENS. CONTROL USE OF LUBRICATING OILS, HYDRAULIC FLUIDS, GREASES, AND OTHER SUCH PRODUCTS. PROMPTLY CLEAN UP AND PROPERLY DISPOSE OF MATERIALS CONTAMINATED BY SPILLAGE OR LEAKAGE OF PRODUCTS.
 7. THE CONTRACTOR SHALL MODIFY AS REQUIRED THE CURRENT APPROVED SWPPP/EROSION CONTROL PLANS FOR EACH PHASE OF THE PROJECT OR AS CONSTRUCTION ACTIVITIES PROGRESS THROUGH THE DURATION OF THE CONTRACT. THESE MODIFICATIONS SHALL BE REPORTED AND COORDINATED WITH BOTH THE QSD AND THE QSP. ANY MODIFICATIONS TO THE OVERALL DURATION OF CONSTRUCTION SHALL BE REPORTED TO THE QSD. THE QSD SHALL THEN BE REQUIRED TO FILE AN EXTENSION OF CONSTRUCTION OR COI, (CHANGE OF INFORMATION), WITH THE STATE WATER RESOURCE CONTROL BOARD. ALL BMP'S SHALL BE MAINTAINED YEAR ROUND TO THE SATISFACTION OF THE QSD AND OSP.
 8. CONTRACTOR SHALL PROTECT ALL EXISTING DRAIN INLETS WITHIN A 500-FT RADIUS FROM THE CENTER OF THE SITE TO PREVENT NON-STORMWATER RUNOFF FROM ENTERING THE STORM DRAIN SYSTEM.
 9. FOR EROSION CONTROL GENERAL NOTES, AND MISCELLANEOUS REQUIREMENTS, SEE DETAIL 1 ON SHEET C-6.1-01.
 10. CONTRACTOR SHALL APPLY SWPPP (STORMWATER POLLUTION PREVENTION PLAN) IF CONSTRUCTION DISTURBED AREA IS EQUAL OR OVER ONE ACRE.
 11. CONTRACTOR SHALL INSTALL TEMPORARY FENCING AROUND THE PERIMETER OF THE CONSTRUCTION SITE AND STAGING AREA. FENCING SHALL BE MINIMUM 8' TALL AND SHALL HAVE A DUST/VISION BARRIER ALONG THE FULL LENGTH. THE DUST/VISION BARRIER SHALL EXTEND THE LENGTH OF THE CONSTRUCTION SITE. THE FENCING SHALL BE ANCHORED TO THE SURFACE AND SHALL BE ABLE TO WITHSTAND A 200-POUND HORIZONTAL POINT LOAD IN ANY DIRECTION. WORK AREA AND GING AREA SHALL BE SECURE AT ALL TIMES.





OVER-EXCAVATION LEGENDS:

REFERENCE: GEOTECHNICAL ENGINEERING REPORT PROJECT NO. 4230.2200060.0000 DATED APRIL 14, 2023 BY UNIVERSAL ENGINEERING SERVICES

IN THE AREA OF THE PROPOSED IMPROVEMENTS, INCLUDING STRUCTURES, ROADWAYS, AND MINOR DISTRESS-SENSITIVE IMPROVEMENTS, EXISTING FILL MATERIAL AND ANY ERODED, DESICCATED, BURROWED, DISTURBED SOILS FROM AGRICULTURAL USE, OR OTHERWISE LOOSE OR DISTURBED SOILS SHOULD BE EXCAVATED TO THE MINIMUM DEPTHS OF SIX FEET IN THE AREAS OF PROPOSED BUILDINGS, TO THE DEPTH OF SUITABLE NATIVE MATERIALS, OR TO A MINIMUM 24 INCHES BELOW THE BOTTOM OF ALL FOOTINGS, WHICHEVER DEPTH IS GREATEST.

PROPOSED PAVEMENT SHOULD BE MOISTURE CONDITIONED TO A DEPTH OF 12 INCHES AND COMPACTED TO ACHIEVE 95 PERCENT COMPACTION.

RECOMPACT 12 INCHES OF SUBGRADE BELOW LANDSCAPING OR PLAY AREAS.

OVEREXCAVATION PLAN

SCALE: 1"=20'

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DATE: 10/02/2023



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o: 949.675.6442

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PROJECT TITLE
**COMPTON COLLEGE
STUDENT HOUSING
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
UNDERGROUND UTILITIES**
1111 E. ARTESIA BLVD., COMPTON, CA 90221



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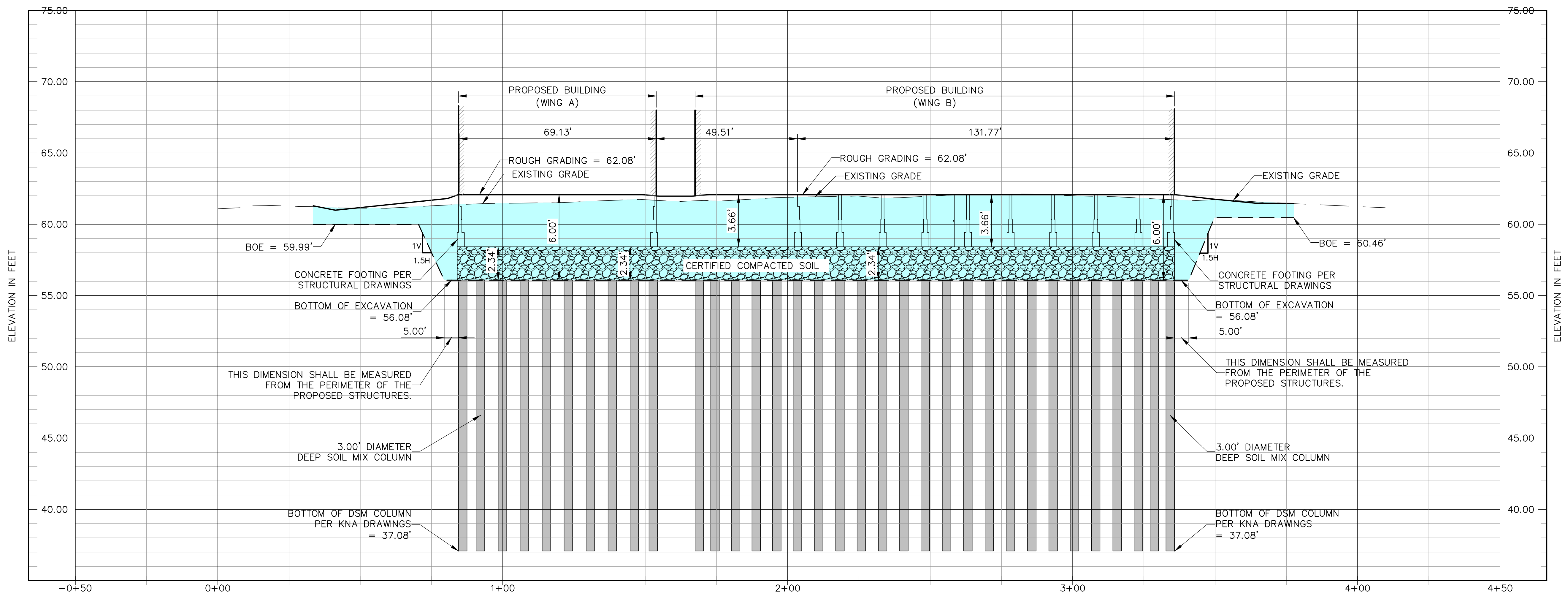
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OVEREXCAVATION PLAN

SHEET NUMBER
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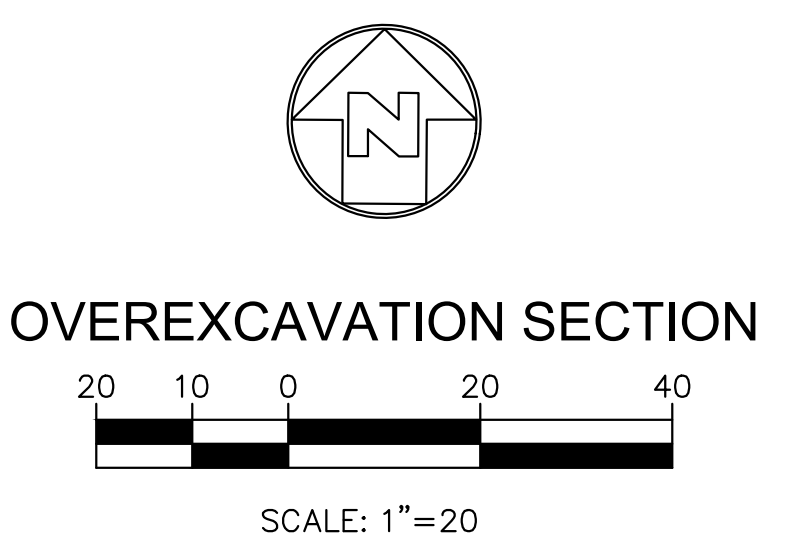
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GRADING SECTION
 SCALE HOR 1"=20' VER 1"=4'

1
 C-7.0

SHEET NOTES:
 1. INSTALL DSM COLUMN UP TO THE ROUGH GRADING SURFACE. OVEREXCAVATE AND CUT PILE UP TO BOTTOM OF EXCAVATION (56.08') DURING INC 2 PER SHEET KNA-3.



PROJECT TITLE
COMPTON COLLEGE STUDENT HOUSING
 INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK & UNDERGROUND UTILITIES
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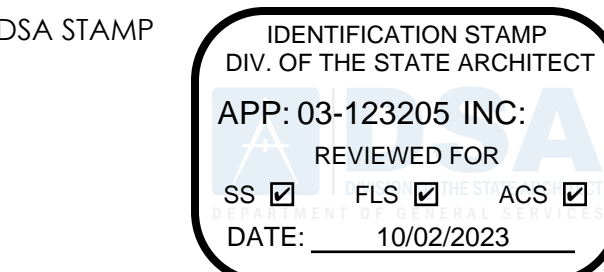
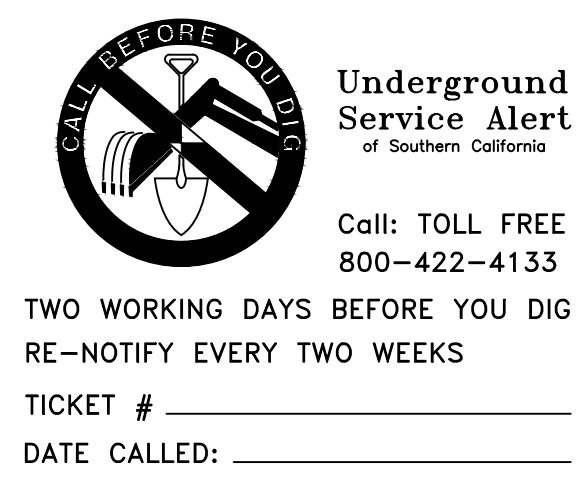
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SHEET TITLE
OVEREXCAVATION SECTION

SHEET NUMBER
C-7.1-01

COMPTON COLLEGE STUDENT HOUSING DEEP SOIL MIXING (DSM)



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GROUND IMPROVEMENT GENERAL NOTES:

- OTHERS are to provide a dry, stable, and relative level working platform. It is Keller North America's (KNA) understanding that the working grade will be near existing grade of El. +57 feet. The working surface shall be constructed and managed by others such that KNA's equipment can safely track and efficiently work under its own weight without the need for steel plates or crane mats.
- The Ground Improvement Engineer is the professional engineer whose stamp resides on this drawing.
- The GENERAL CONTRACTOR shall confirm that the proposed operation does not conflict with future improvement such as structural, mechanical, plumbing, and electrical prior to DSM installation.
- An underground service alert must be obtained 2 days before starting work.
- All permits shall be procured and paid for by the OWNER, other than transportation permits required for KNA's mobilization and demobilization.
- All encroachment permits within the public right of way and letters of permission from private owners must be obtained by the OWNER.
- KNA will provide a qualified full-time quality control (QC) representative. This representative is either KNA's Superintendent/Foreman/or Field Engineer. Third party testing and/or inspection shall be provided by OTHERS.
- Locating, protecting and rerouting/removal of all utilities are the responsibility of OTHERS. KNA is not responsible for damage to existing utilities.
- After the completion of Ground Improvement work, OTHERS are responsible for the protection of DSM columns. Proper site drainage to prevent ponding of water at the area of the soil-mixed columns and control coordination of earthwork activities shall be managed such that existing soil-mixed columns are not damaged.
- The DSM locations shown on the approved construction drawings are only for Ground Improvement layouts. These plans should not be used for foundation layout.
- All post-improvement testing including frequency and criteria for soil-mixed columns are noted on the plans and design submittal.
- Foundations shall not be poured until approved by the project Geotechnical Engineer of Record.
- Alternate structural shapes, material, and details cannot be used unless reviewed and approved by the Ground Improvement Design Engineer, DSA & CGS.
- DSM to provide allowable static soil bearing pressure of up to 2,000 psf. Allow for a 1/3 increase for transient loads such as wind/seismic loading.
- DSM to provide a coefficient of friction of 0.35₂.
- DSM to provide post-construction total static settlement of less than 1 inch.
- DSM to provide post-construction total liquefaction settlement of less than 1 inch.
- Max differential settlement of less than 1 inch over 13.9 feet.
- The drawing set is based on KNA's DSM design submittal REV 01 dated 07/03/2023 and the final geotechnical report provided by Universal Engineering Sciences, Project No. 4230.2200060.0000 dated 07/03/2023.
- All DSM columns have been arranged to achieve a minimum of 35% Area Replacement Ratio (ARR) under all foundational elements.

DSM VERIFICATION NOTES:

- The acceptance of the work shall be based on demonstrating that the in-place mixing of grout with the treatment soils has achieved the average design strength requirements. Soilcrete strengths shall be determined statistically by wet (grab) sample and core samples. Confirmation sample collection and testing will be conducted by KNA. Samples shall be collected by KNA using wet sampling and continuous core sampling techniques described below. Test shall be performed at the frequencies described below. Sample collection perform by KNA, testing will be performed by lab hired by owner.
- Wet Soil mix samples will be retrieved and cast into molds for one column per rig/shift, at one random depth, typically near the end of each shift. Samples will be retrieved using an in situ wet sampler immediately after column construction and shall consist of no fewer than 8 specimens. These samples shall be tested in pairs: two at seven (7) days, two at fourteen days (14), two at twenty eight (28) days and two at fifty six (56) days if necessary. Soil clouds greater than 10% of the mold diameter will be screened off. Appropriate curing techniques shall be implemented until testing based on ASTM D 1632.
- Unconfined compression testing shall be performed by an approved laboratory working directly for the OWNER. Samples shall be tested in pairs starting at 7-days. If the 7-day specimens do not reach the desired strength according to the lab test curve, another pair of specimens will be tested at 14 days, 28 days, and if needed at 56 days. All specimens at 28 days and available 56-days of age will be tested and used in the statistical calculation.
- If wet grab strengths at 7 days of age are greater than the average required (150 psi) unconfined compressive strength, additional tests may be omitted at the discretion of the GEOR. Wet grab samples will be kept on-site (approximately 3 days) for an initial set before being shipped to the lab.
- The Unconfined Compressive Strength (UCS) shall be determined by ASTM D1633 "Standard Test Methods for Compressive Strength of Molded Soil-Cement Cylinders". Sulfur or gypsum end caps shall be required in the UCS tests to minimize the end effects on the test specimen. The advantage of the wet sampling is that KNA can get an early trend of the soilcrete strength development without waiting to the end of the project for coring and can make early decisions in the field program to add additional soil mixing columns if necessary.
- KNA will core 2% of the production DSM columns.
- All core locations shall be randomly selected and the selection of locations for confirmation coring and selection of core samples for UCS testing are subject to review and approval of the Geotechnical Engineer of Record (GEOR) for the project.

- At minimum five (5) samples from each core will be extracted. KNA anticipates 5 specimens trimmed from each core hole and tested by ASTM D1633.
- KNA will calculate the average 28-day UCS value from all core samples and wet grab samples. The target average 28 days UCS value shall be 150 psi or greater. Averages will be taken together.
- No more than 10 percent of all specimens tested shall exhibit an unconfined compressive strength of less than 75psi at 28 day of age.
- If the acceptance criteria is not achieved in a designated area, KNA may be given the opportunity to conduct additional UCS test on soilcrete specimens on 56 days of age, site exploration, coring, sampling, downhole imaging, and strength testing from the additional cured specimen to better define the average design strength at KNA's preference and expense. If a designated area is rejected, KNA shall submit a Remixing or Mitigation plan.
- Uniformity of mixing shall be evaluated by the Ground Improvement Design Engineer and the Geotechnical Engineer of Record (GEOR) based on the continuous core samples recovered. The continuous core holes shall extend the entire depth of the DSM column. Estimated recovery of 85 percent for each 5-foot-long segment of a boring and at least 85 percent when averaged over all core runs within a single boring shall be achieved. The lumps of unimproved soils shall not exceed 15 percent of the total volume of any 5-foot core segment from a boring. If the core recovery is below the anticipated value, KNA shall be allowed to utilize a downhole camera or other approved methods to verify the core hole. This may include additional cores in the same column.
- At the end of the project, to not unnecessary delay subsequent activities by waiting for 28 days test result, a correction of early strength gain will be used to approve the soil-mixed column work. However, this correlation will not relieve the contractor of the responsibility to achieve average 28 days strength. Based on FHWA (2013) guidelines, the following UCS aging factor correlations will be applied to this job:
 - 7 day to 28 day projection factor: 1.35
 - 14 day to 28 day projection factor: 1.15
- A site-specific correlation between 3 days and 28 days strength may be used to supersede this correlation if in the opinion of the Engineer, the site-specific correlation is more appropriate.
- Special inspection of soil improvement work is required by the project geotechnical engineer listed on the form DSA 1 (or assuming fill responsibility through form DSA 109). This geotechnical engineer shall not be employed by the contractor or ground improvement sub-contractor per CAC 4-335(f). The geotechnical engineer performing special inspection shall submit a final verified report (form DSA 293) covering all geotechnical aspects of the project subject to special inspections, inclusive of the soil improvement work. Special inspection is not a substitute for nor change quality control requirements.
- After completion of the recommended and accepted final ground improvement program, the consultants should provide a comprehensive final report for CGS review. The report should document their observations, testing, and analysis, including the data collected to satisfy the specified acceptance criteria. The report should include (at a minimum):
 - All DSM installation logs/records, field testing records, as-built plan and record of installed DSM elements, and daily field reports from both the contractor and consultants' field representative(s).
 - All equipment calibration reports, QA/QC data and records of DSM installation data.
 - All DSM coring logs, any downhole televiewer logs, and laboratory test results, including summary and calculations of the UCS values of the DSM elements.
 - Any other pertinent data gathered and/or observations made during the performance of the ground improvement program that are considered in assessing the satisfaction of the design objectives.
 - Discussion and conclusion(s) regarding satisfaction of the DSM design and performance requirements for the project.

DSM CONSTRUCTION:

- OWNER will provide to KNA, at least four (4) control points. KNA will provide an AutoCAD Shop Drawing for all DSM columns overlaid on the site Civil drawing and stake all DSM locations.
- DSM columns will be installed within 3 inches of the design locations as shown in the KNA shop drawing. Construction tolerances:
 - Plan location ±3 inches
 - Verticality ±1% of plumb
- Modifications of DSM locations, diameter, or depth shall be approved by KNA design engineer and GEOR. Additionally, a CCD containing the revisions shall be submitted to DSA for review and approval. KNA retains the sole authority to modify DSM column locations due to constructability and/or site constraints. KNA will prepare as-built drawings after completion.
- Once a stable working platform has been established as shown in KNA Shop Drawing. DSM columns will be constructed sequentially based on a pattern dictated in the Field. KNA requires access to all DSM locations at all times to maximize efficiency.
- To minimize the mixing tool damage and maintaining soil mixing quality, KNA may pre-dill holes or excavate for better mixing quality. The holes will be filled with soilcrete up to the working elevation of +62 feet during the mixing stage.
- In general, soil mixing operation parameters, such as mixing shaft speed, penetration rate, batching grout specific gravity, and pumping rate will be determined based on our lab mixing results and our experience and will be fine-tuned at the beginning of mixing column production. The design cement content in place (cement weight/[soil volume + grout volume]) will start from predetermined cement content and grout slurry specific gravity (sg). KNA's Engineers may adjust the cement content and specific gravity based on the field sample strength development.
- Vertical alignment of the mix tool stroke will be controlled by the drill rig operator. Two measurements of verticality will be monitored. These are the fore-aft and left-right vertical mast positions. Verticality will be measured by a level as measured on the mixing tool prior to penetration. Intermittent measurements will be made as may be necessary during mixing operations.
- The mixing shaft speed which is anticipated to be ranging between 40-60 RPM and shall be adjusted to accommodate a constant rate of mixing shaft penetration based on the degree of drilling difficulty. The mixing shaft speed can be adjusted according to drilling difficulty. The mixing shaft speed can be adjusted to aid mixing

- of the soil column when needed or to assist penetration in hard drilling. Mixing shaft speed will be recorded.
- In order to ensure adequate mixing, the penetration rate of the mixing shaft shall be maintained at about 1.0 to 3.0 feet/minute during penetration but will vary based on actual site conditions. The penetration rate and maximum depth of each stroke shall be recorded by KNA's data acquisition system (DAQ).
 - The grout slurry (with specific gravity ranging from 1.36 to 1.55) flow per vertical foot of the column will be adjusted to the requirements of the design mix. Progressive cavity pumps will be used to transfer the grout from the mixing plant to the mixing rig. Flow monitoring devices will be installed in the grout line to detect any line blockage and monitor flow, total injected grout per column and grout pressure. These parameters will be recorded.
 - Inevitably some variations of the grout take will occasionally occur due to field conditions. It is anticipated that a grout flow rate between 20 to 160 GPM will be used during penetration. KNA's Data Acquisition System (DAQ) can automatically adjust the grout flow rate as a function of the penetration rate and maintain the pre-set cement dosage prescribed by the design engineer.
 - The mixing shaft will be withdrawn at a rate of 6 to 12 feet per minute during the re-stroke operation and complete removal of the mixing shaft from the ground thus mixed.
 - KNA will use a data acquisition system to monitor the mixing shaft penetration and the shaft rotation resistance in terms of the hydraulic pressure. The DAQ system will calculate and plot the Drilling Index as a function of depth, a mixing parameter to detect penetration resistance and refusal depth. KNA will set up the penetration criteria based on the site measurement. In case of underground obstruction, such as abandoned footings, piles, utilities, etc., the general contractor will be responsible to remove obstructions and backfilled with sandy soil prior KNA mixing operation.
 - Cement will be furnished by KNA and conform to ASTM C150 "Standard Specification for Portland Cement," Type II/V or equivalent. The cement will be adequately protected from moisture and contamination while in transit to and in storage at the job site. Reclaimed cement or cement containing lumps or deleterious matter will not be used.
 - Water for the slurry will be fresh, free of deleterious substances that adversely affect the strength and mixing properties of the slurry, furnished by the OTHERS.
 - The batch plant shall consist of in-line eductor (jet valve) mixers. Dry materials shall be stored in tankers and/or silos and fed to the mixers for shearing and circulation. The resulting grout slurry will be transferred to a surge tank for continuous agitation and to supply the in-situ soil mixing rig. Grout slurry quality will be assured by frequent testing prior to injection into the soil.
 - Single shaft mixing equipment that mechanically mixes the soil and cement slurry for the full dimensions of the column will be used for the work. We anticipate using hydraulic drill rigs for the soil mixing operations. This rig is capable of up to > 150,000 ft-lbs. of torque at > 20 rpm. The working shaft rate of rotation ranges between 20 and 60 rpm. The mixing shaft will have mixing augers and/or blades (paddles) configured in such a manner so that they are capable of thoroughly blending the in-situ soils and cement slurry. The power source for driving the mixing shafts will be sufficient to maintain the required mix tool (shaft) rotation speed in revolutions per minute and penetration/ withdrawal rates from the ground surface to the maximum depth required. The design target Blade Rotation Number (BRN, defined as the number of blades cut in each 1.0-meter soil) will be at least 300.
 - The DSM equipment will be equipped with devices to assure vertical alignment in two planes (90 degrees in plan from each other): fore-aft and left-right. The DSM equipment will be equipped with a real-time display of depth, rotation speed, grout flow rate; grout specific gravity, cumulative grout injected, and grout pressure for each soil mix column. The cement will be mixed with water within the jet valve to create a 1.45 specific gravity mix +/- 0.1. No mixing operation will be only allowed if the DAQ system not functioning.
 - Grout slurry will be supplied to the drill using large size Moyno pumps. These pumps will be sized and powered so that design volumes and pressures can be maintained up to 1,000 ft away from the batching facility. It is anticipated that a continuous grout slurry flow of 150 gallons per minute at 100 psi to the drill rig will be necessary.
 - The batching and pumping facility will be set up at a central location to areas all structures. This will eliminate the need to move the plant once it is established.

| DRAWING SHEET INDEX | |
|----------------------------------|--------------|
| SHEET NAME | SHEET NUMBER |
| TITLE PAGE - DSM GENERAL NOTES | KNA-1 |
| OVERALL DEEP SOIL MIXING LAYOUT | KNA-2 |
| TYPICAL DEEP SOIL MIXING DETAILS | KNA-3 |



SEAL

CONSULTANTS

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909-393-9300

PROJECT TITLE
**COMPTON COLLEGE
STUDENT HOUSING**
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD, COMPTON, CA 90221



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| 1 | 09/05/2023 | DSA BACKCHECK SUBMITTAL |
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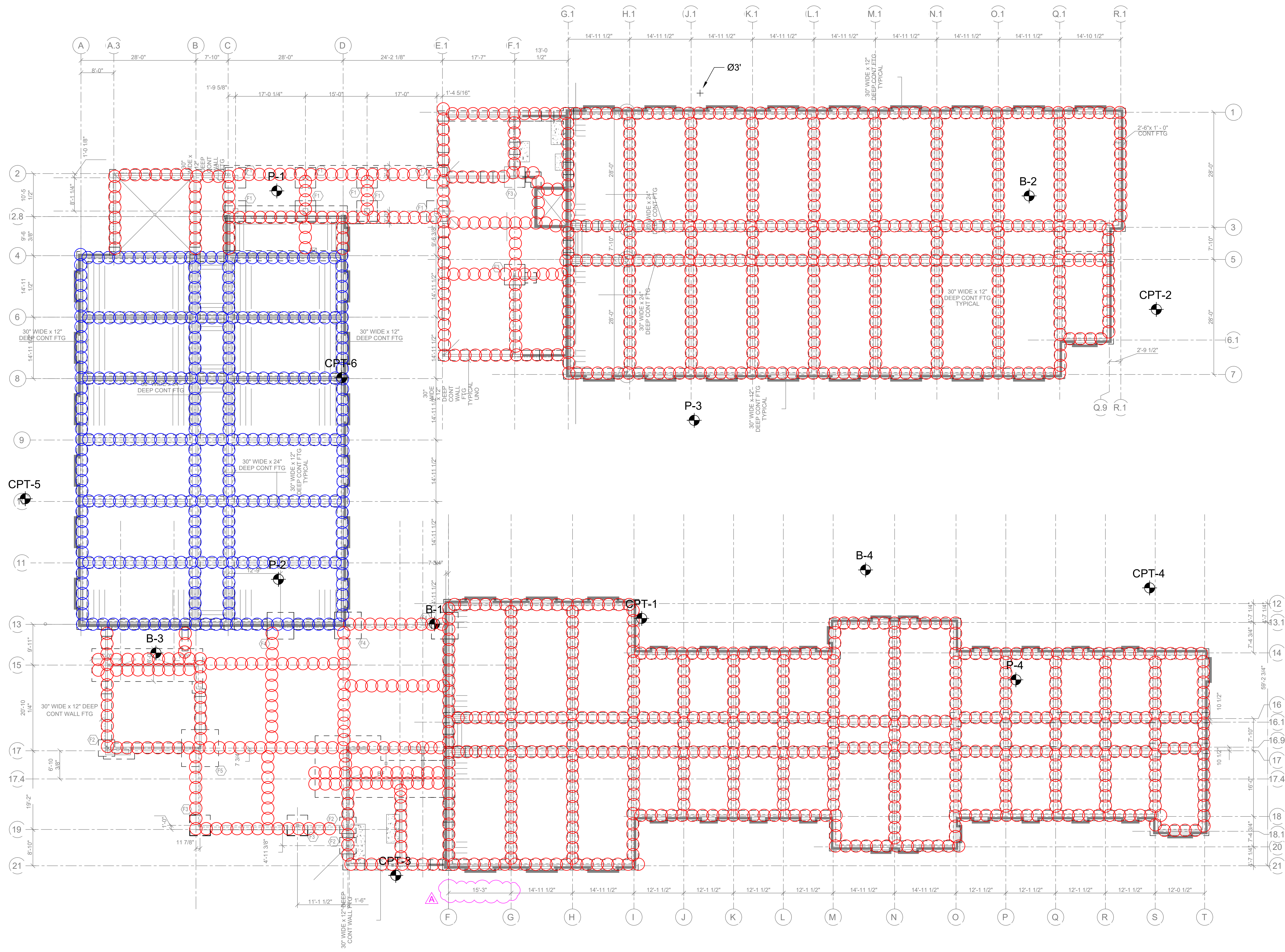
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SHEET TITLE
**TITLE PAGE -
DSM GENERAL NOTES**

SHEET NUMBER

KNA-1

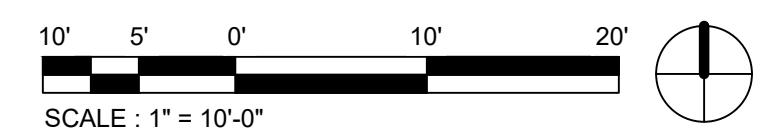
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LEGEND:

- 3 foot diameter column, treatment depth of 20 feet from El. +62 feet
The total length of the DSM column should be verified during construction by the actual depth of coarse-grained materials
- 3 foot diameter column, treatment depth of 25 feet from El. +62 feet
The total length of the DSM column should be verified during construction by the actual depth of coarse-grained materials

● Boring/CPT Locations



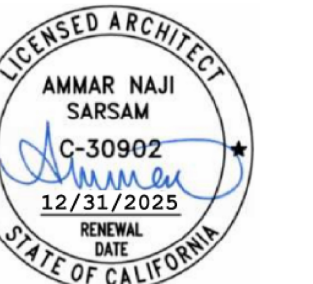
OVERALL DEEP SOIL MIXING LAYOUT SCALE 1" = 10'-0" **2**

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PROJECT TITLE
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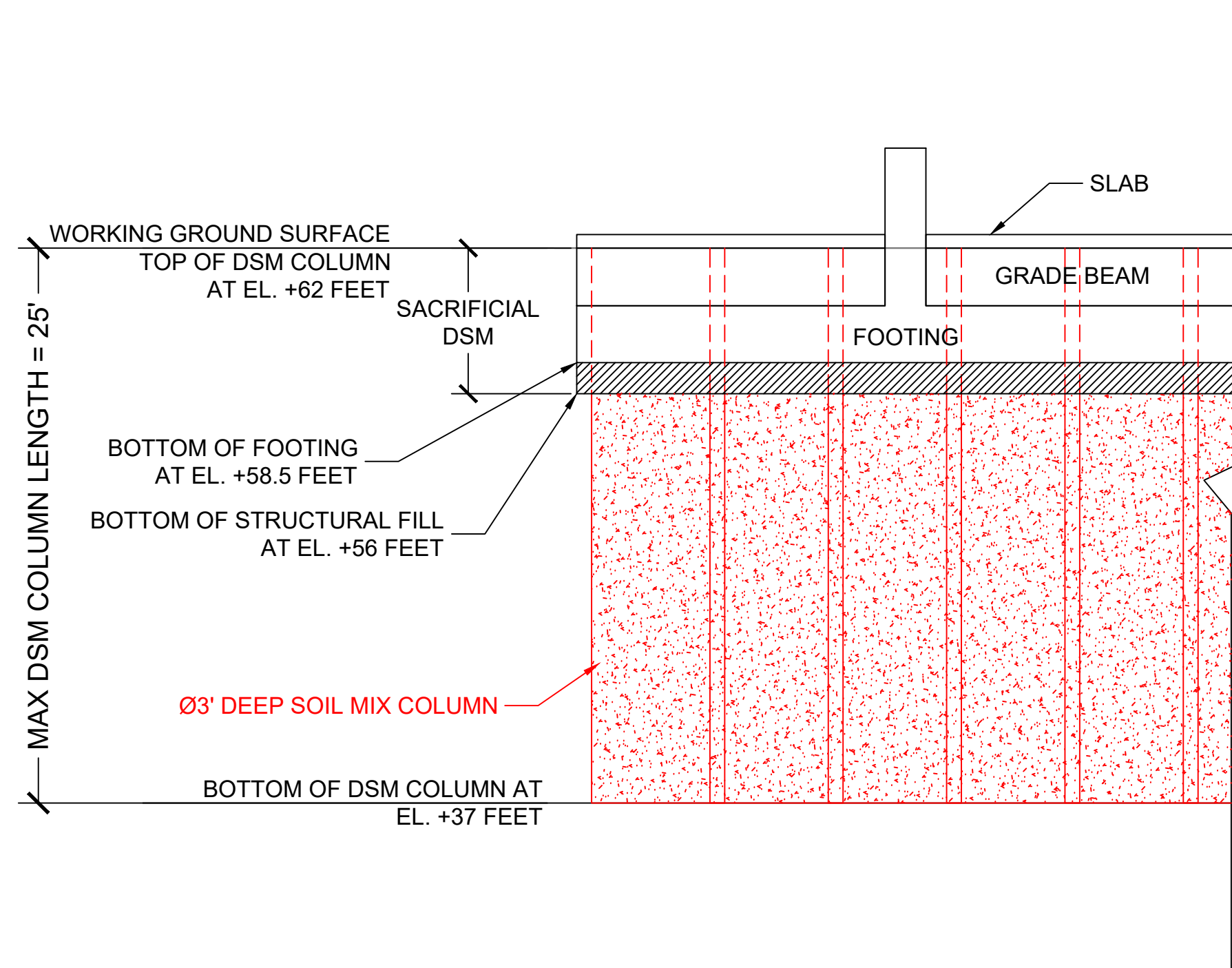
SHEET TITLE
OVERALL DEEP SOIL
MIXING LAYOUT

SHEET NUMBER

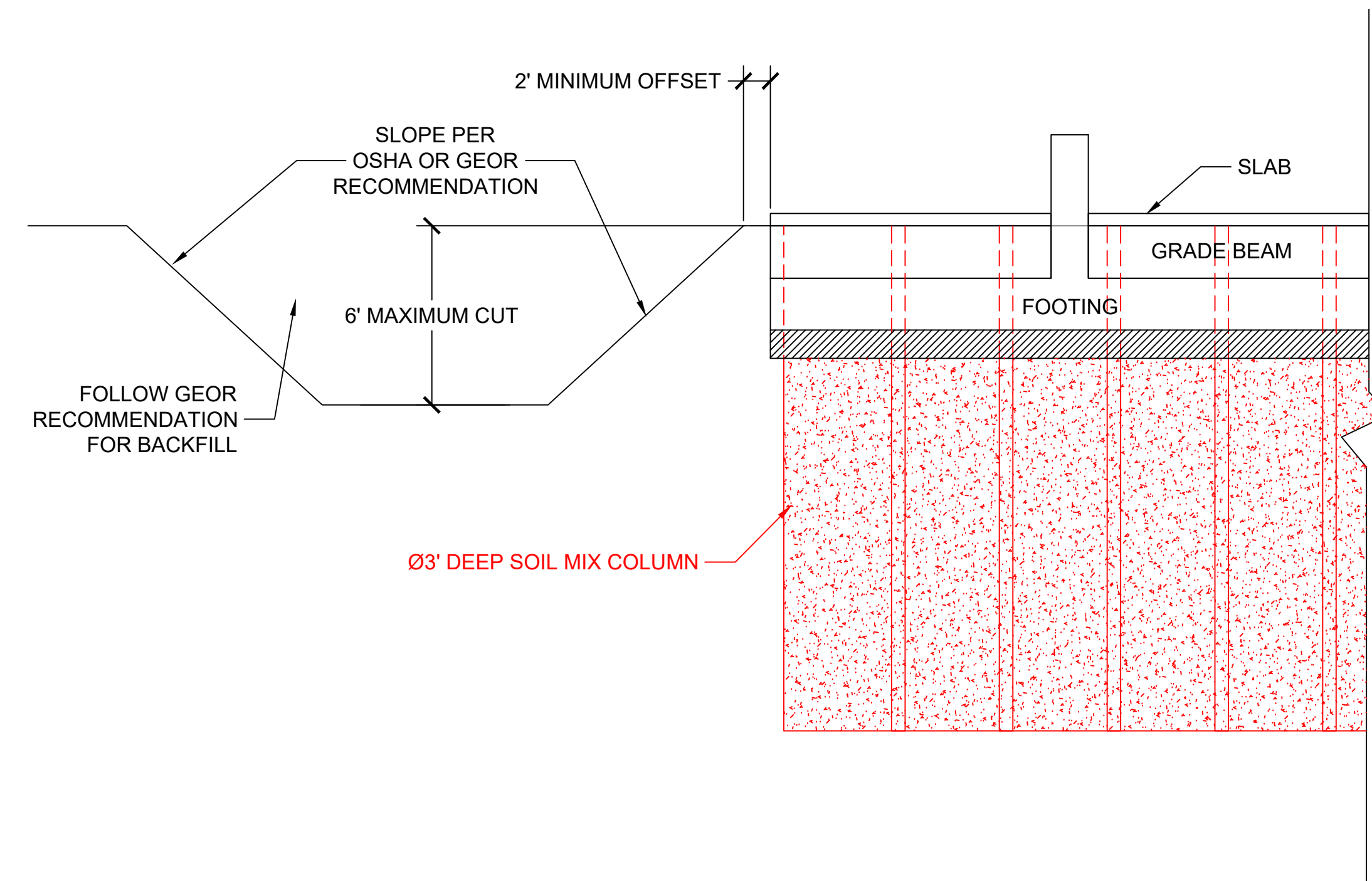
KNA-2

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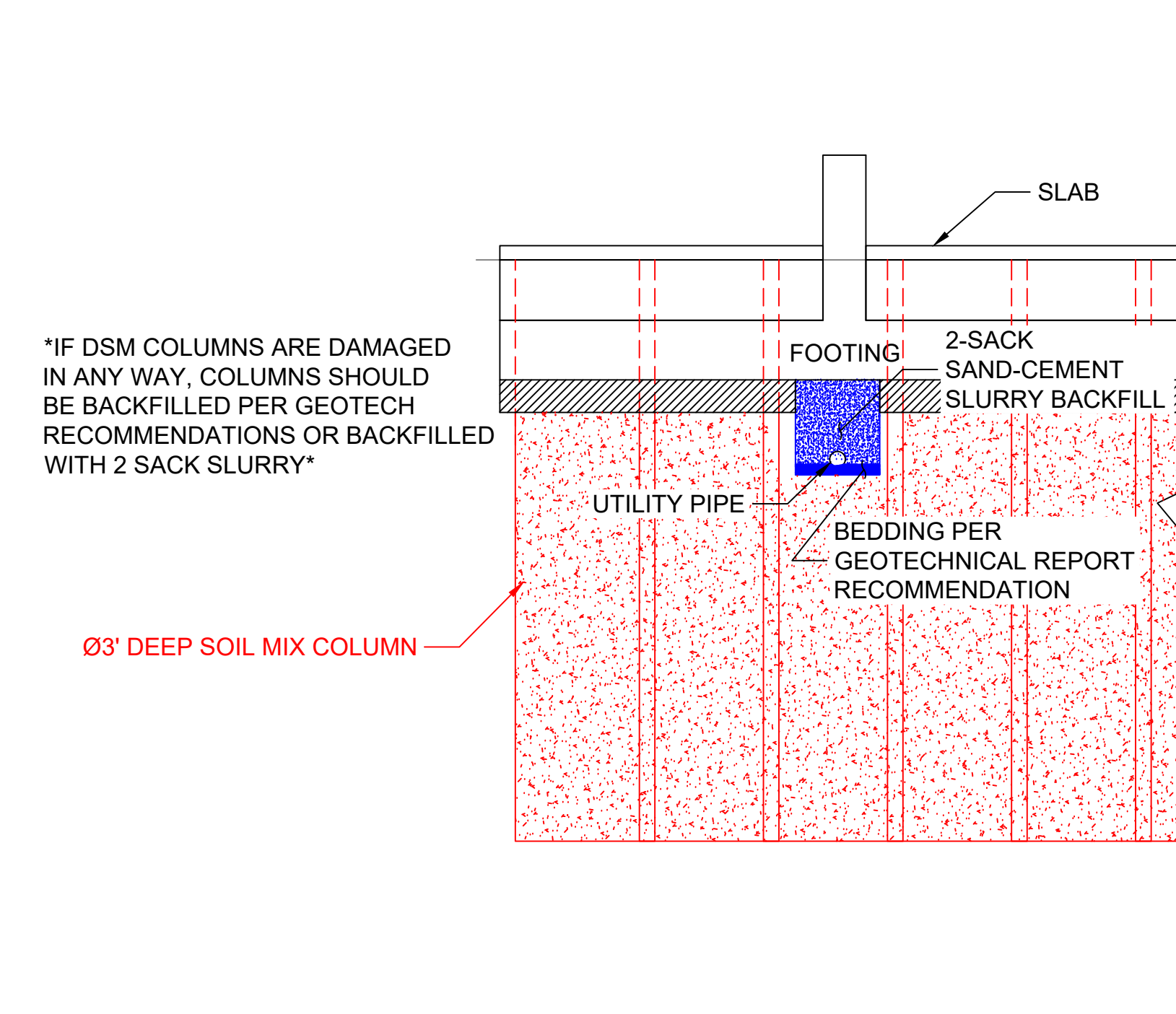
TYPICAL DEEP SOIL MIX COLUMN DETAIL
(NOT TO SCALE)



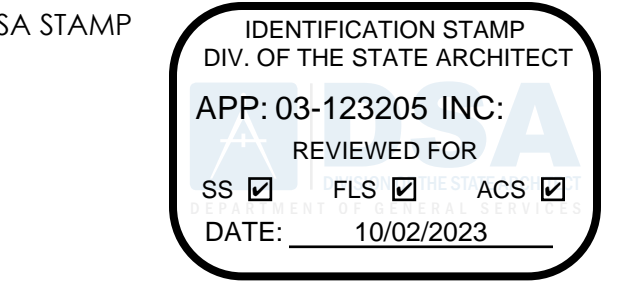
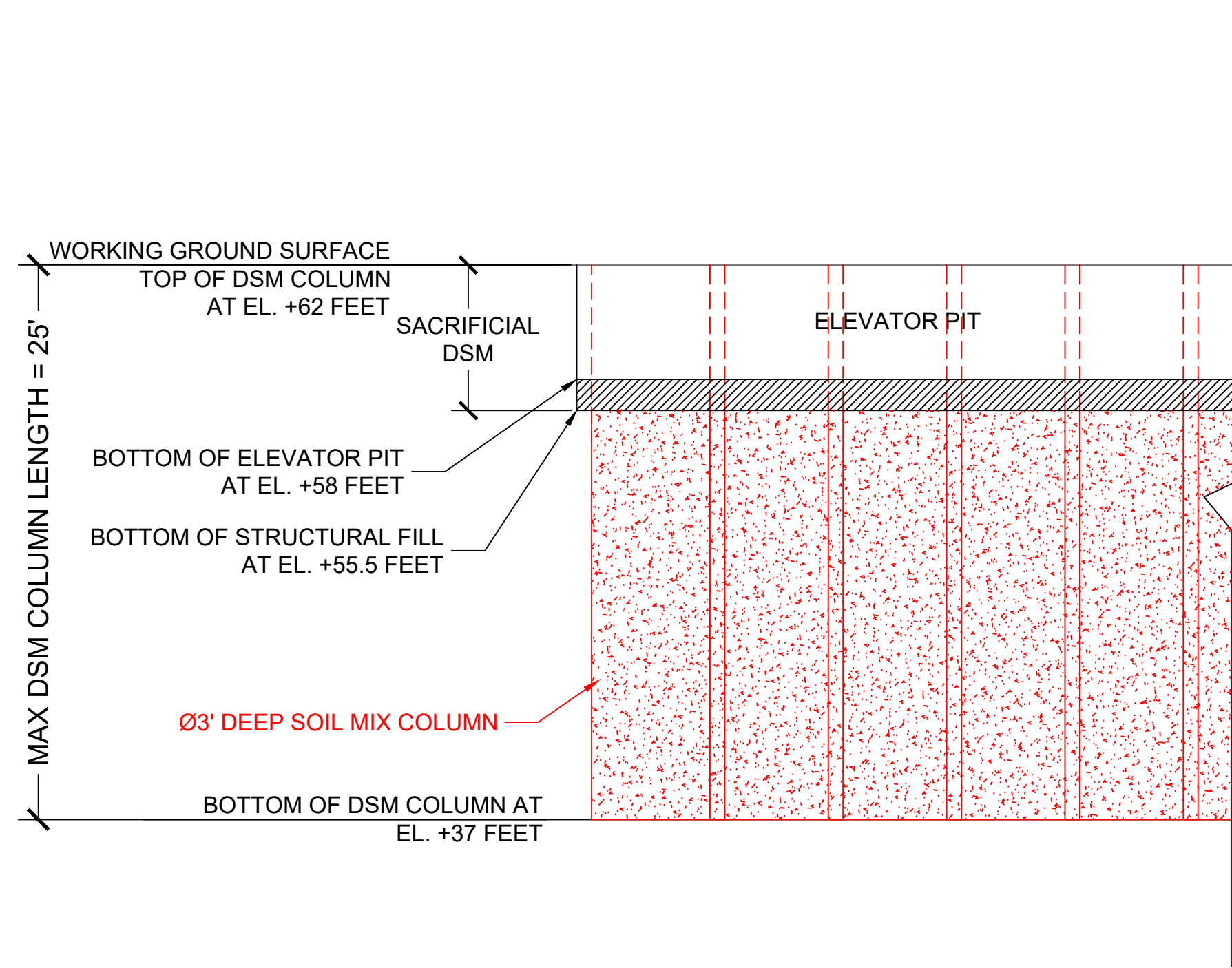
TYPICAL UTILITY EXCAVATION DETAIL
(NOT TO SCALE)



TYPICAL UTILITY TRENCH THROUGH DSM COLUMN
(NOT TO SCALE)



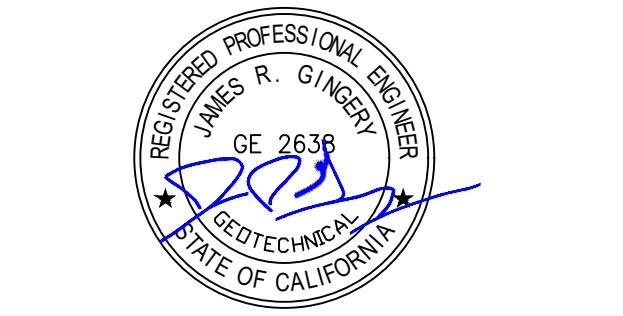
DEEP SOIL MIX COLUMN ELEVATOR PIT DETAIL
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PROJECT TITLE
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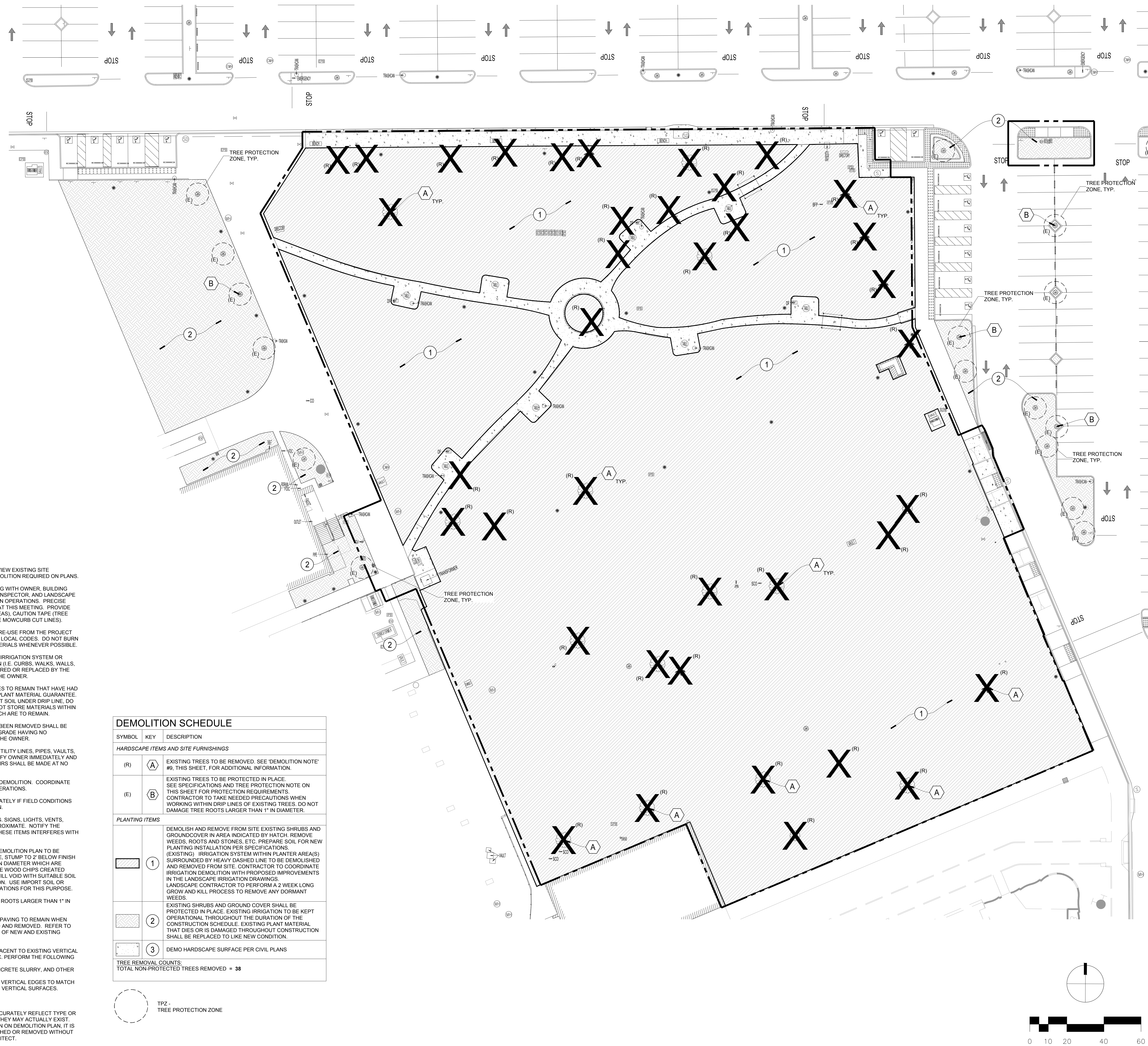
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SHEET TITLE
TYPICAL DEEP SOIL MIXING DETAILS

SHEET NUMBER

KNA-3



DEMOLITION NOTES

- PRIOR TO SUBMITTING BID, VISIT SITE TO REVIEW EXISTING SITE CONDITIONS AND TO VERIFY EXTENT OF DEMOLITION REQUIRED ON PLANS.
- SCHEDULE A DEMOLITION "KICK-OFF" MEETING WITH OWNER, BUILDING OPERATIONS, GENERAL CONTRACTOR, CITY INSPECTOR, AND LANDSCAPE ARCHITECT PRIOR TO BEGINNING DEMOLITION OPERATIONS. PRECISE LIMITS OF DEMOLITION WILL BE CONFIRMED AT THIS MEETING. PROVIDE SPRAY PAINT CANS (MARK PAVING DEMO AREAS), CAUTION TAPE (TREE DEMO), AND BLUE MASKING TAPE (CONCRETE MOWCURB CUT LINES).
- REMOVE ITEMS NOT REQUIRED FOR FILL OR RE-USE FROM THE PROJECT SITE AND DISPOSE OF IN ACCORDANCE WITH LOCAL CODES. DO NOT BURN RUBBISH OR DEBRIS ON SITE. RECYCLE MATERIALS WHENEVER POSSIBLE.
- ANY DAMAGE TO EXISTING PLANT MATERIAL, IRRIGATION SYSTEM OR HARDSCAPE ELEMENTS THAT ARE TO REMAIN (I.E. CURBS, WALKS, WALLS, ADJACENT PROPERTY, ETC.) SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
- CONTRACTOR SHALL LIST ALL EXISTING TREES TO REMAIN THAT HAVE HAD WORK PERFORMED WITHIN 8' OF TRUNK ON PLANT MATERIAL GUARANTEE. PROTECT TREES IN PLACE. DO NOT COMPACT SOIL UNDER DRIP LINE, DO NOT PARK VEHICLES WITHIN DRIP LINE, DO NOT STORE MATERIALS WITHIN DRIP LINE, OR OTHERWISE HARM TREES WHICH ARE TO REMAIN.
- ALL SURFACES WHERE GROUND COVER HAS BEEN REMOVED SHALL BE GRUBBED AND BROUGHT TO A CONSISTENT GRADE HAVING NO IRREGULARITIES, TO THE SATISFACTION OF THE OWNER.
- VERIFY LOCATIONS OF ALL UNDERGROUND UTILITY LINES, PIPES, VAULTS, OR BOXES PRIOR TO ANY EXCAVATION. NOTIFY OWNER IMMEDIATELY AND REPAIR ANY SUCH ITEMS IF DAMAGED. REPAIRS SHALL BE MADE AT NO ADDITIONAL COST TO THE OWNER.
- SAFE-OFF UTILITIES AS REQUIRED PRIOR TO DEMOLITION. COORDINATE SERVICE INTERRUPTIONS WITH BUILDING OPERATIONS.
- CONTRACTOR SHALL NOTIFY OWNER IMMEDIATELY IF FIELD CONDITIONS SHOULD VARY FROM THOSE SHOWN ON PLAN.
- LOCATIONS OF EXISTING SITE ELEMENTS (E.G. SIGNS, LIGHTS, VENTS, HYDRANTS, TRANSFORMERS, ETC.) ARE APPROXIMATE. NOTIFY THE OWNER IMMEDIATELY IF THE LOCATION OF THESE ITEMS INTERFERES WITH THE PROPER EXECUTION OF WORK.
- WHEN DEMOLISHING TREES INDICATED ON DEMOLITION PLAN TO BE REMOVED BY AN "(R)" SYMBOL, REMOVE TREE, STUMP TO 2' BELOW FINISH GRADE, AND ROOTS GREATER THAN 1-INCH IN DIAMETER WHICH ARE LOCATED IN TOP 12-INCHES OF SOIL. REMOVE WOOD CHIPS CREATED FROM STUMP GRINDING PROCESS, THEN REFILL VOID WITH SUITABLE SOIL AND COMPACT TO 80% RELATIVE COMPACTION. USE IMPORT SOIL OR EXCESS SITE SOIL AS INDICATED IN SPECIFICATIONS FOR THIS PURPOSE.
- WHEN REMOVING PLANT MATERIAL, REMOVE ROOTS LARGER THAN 1" IN DIAMETER.
- PROVIDE CLEAN SAWCUT EDGE AT EXISTING PAVING TO REMAIN WHEN EXISTING CONCRETE PAVING IS DEMOLISHED AND REMOVED. REFER TO HARDSCAPE PLAN AND DETAILS FOR JOINING OF NEW AND EXISTING PAVING.
- WHEN DEMOLISHING CONCRETE PAVING ADJACENT TO EXISTING VERTICAL SURFACES, I.E. BUILDING, WALLS, STEPS, ETC. PERFORM THE FOLLOWING PROCEDURES:
 - REMOVE EXISTING JOINT CAULKING, CONCRETE SLURRY, AND OTHER DELETERIOUS MATERIALS.
 - AFTER INSTALLING NEW PAVING, REPAIR VERTICAL EDGES TO MATCH ADJACENT SECTIONS OF NON-EFFECTED VERTICAL SURFACES.
- DO NOT BURY VEGETATION.
- THIS DEMOLITION PLAN MAY OR MAY NOT ACCURATELY REFLECT TYPE OR EXTENT OF ITEMS TO BE ENCOUNTERED AS THEY MAY ACTUALLY EXIST. WHERE EXISTING FEATURES ARE NOT SHOWN ON DEMOLITION PLAN, IT IS NOT IMPLIED THAT THEY ARE TO BE DEMOLISHED OR REMOVED WITHOUT PRIOR AUTHORIZATION BY LANDSCAPE ARCHITECT.

| DEMOLITION SCHEDULE | | |
|--|-----|---|
| SYMBOL | KEY | DESCRIPTION |
| HARDSCAPE ITEMS AND SITE FURNISHINGS | | |
| (R) | A | EXISTING TREES TO BE REMOVED. SEE 'DEMOLITION NOTE' #9, THIS SHEET, FOR ADDITIONAL INFORMATION. |
| (E) | B | EXISTING TREES TO BE PROTECTED IN PLACE. SEE SPECIFICATIONS AND TREE PROTECTION NOTE ON THIS SHEET FOR PROTECTION REQUIREMENTS. CONTRACTOR TO TAKE NEEDED PRECAUTIONS WHEN WORKING WITHIN DRIP LINES OF EXISTING TREES. DO NOT DAMAGE TREE ROOTS LARGER THAN 1" IN DIAMETER. |
| PLANTING ITEMS | | |
| 1 | | DEMOLISH AND REMOVE FROM SITE EXISTING SHRUBS AND GROUND COVER IN AREA INDICATED BY HATCH. REMOVE WEEDS, ROOTS AND STONES, ETC. PREPARE SOIL FOR NEW PLANTING INSTALLATION PER SPECIFICATIONS. (EXISTING) IRRIGATION SYSTEM WITHIN PLANTER AREA(S) SURROUNDED BY HEAVY DASHED LINE TO BE DEMOLISHED AND REMOVED FROM SITE. CONTRACTOR TO COORDINATE IRRIGATION DEMOLITION WITH PROPOSED IMPROVEMENTS IN THE LANDSCAPE IRRIGATION DRAWINGS. LANDSCAPE CONTRACTOR TO PERFORM A 2 WEEK LONG GROW AND KILL PROCESS TO REMOVE ANY DORMANT WEEDS. |
| 2 | | EXISTING SHRUBS AND GROUND COVER SHALL BE PROTECTED IN PLACE. EXISTING IRRIGATION TO BE KEPT OPERATIONAL THROUGHOUT THE DURATION OF THE CONSTRUCTION SCHEDULE. EXISTING PLANT MATERIAL THAT DIES OR IS DAMAGED THROUGHOUT CONSTRUCTION SHALL BE REPLACED TO LIKE NEW CONDITION. |
| 3 | | DEMO HARDSCAPE SURFACE PER CIVIL PLANS |
| TREE REMOVAL COUNTS: TOTAL NON-PROTECTED TREES REMOVED = 38 | | |

TPZ - TREE PROTECTION ZONE

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 ISSUED: 07/21/24
 EXPIRES: 07/21/2027
 STATE OF CALIFORNIA

CONSULTANTS
 RIA
 REGISTERED LANDSCAPE ARCHITECT
 JESSIE EBERHART
 C-30902
 ISSUED: 07/21/24
 EXPIRES: 07/21/2027
 STATE OF CALIFORNIA
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 IRVINE - CA 92618
 949.387.1323
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PROJECT TITLE
**COMPTON COLLEGE
 STUDENT HOUSING**
 INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
 UNDERGROUND UTILITIES
 1111 E. ARTESIA BLVD., COMPTON, CA 90221



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SHEET TITLE
**LANDSCAPE
 DEMOLITION PLAN**

SHEET NUMBER

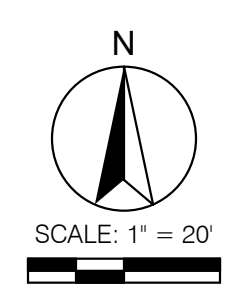
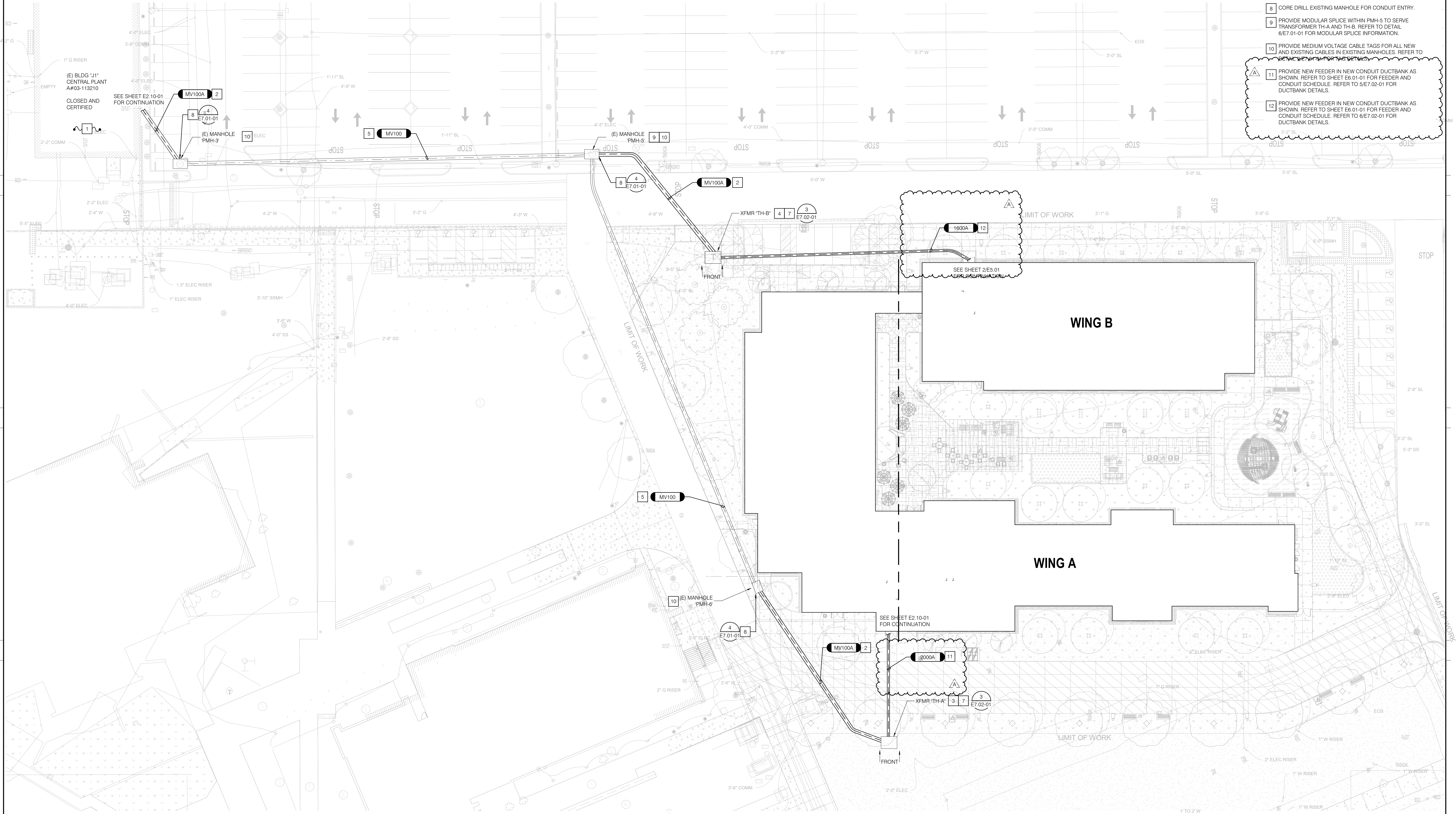
L1.01
 CONSTRUCTION DOCUMENTS

GENERAL NOTES

A. REFER TO SINGLE LINE DIAGRAM ON SHEET E6.01-01 FOR FEEDER SIZING AND EQUIPMENT CONNECTION DETAILS.

NOTES

- 1 CAMPUS POINT OF SERVICE FOR MEDIUM VOLTAGE IS LOCATED AT SITE SWITCH 'PMS-7'.
- 2 PROVIDE NEW FEEDER IN NEW CONDUIT DUCTBANK AS SHOWN. REFER TO SHEET E6.01-01 FOR FEEDER AND CONDUIT SCHEDULE. REFER TO 2/E7.02-01 FOR DUCTBANK DETAILS.
- 3 PROVIDE 750KVA LIQUID-FILLED PADMOUNT TRANSFORMER TH-A ALONG WITH ALL ASSOCIATED ACCESSORIES AND SECONDARY BREAKERS IN NEMA 3R ENCLOSURES AS SHOWN. PROVIDE 8'X10' CONCRETE PAD. PROVIDE MINIMUM 8' CLEARANCE IN FRONT OF TRANSFORMER CABINET FACE. REFER SINGLE LINE DIAGRAM SHEET E6.01-01 FOR TRANSFORMER RATING AND ADDITIONAL INFORMATION.
- 4 PROVIDE 500KVA LIQUID-FILLED PADMOUNT TRANSFORMER TH-B ALONG WITH ALL ASSOCIATED ACCESSORIES AND SECONDARY BREAKERS IN NEMA 3R ENCLOSURE AS SHOWN. PROVIDE 8'X10' CONCRETE PAD. PROVIDE MINIMUM 8' CLEARANCE IN FRONT OF THE TRANSFORMER CABINET FACE. REFER TO SINGLE LINE DIAGRAM SHEET E6.01-01 FOR TRANSFORMER RATING AND ADDITIONAL INFORMATION.
- 5 PROVIDE NEW CONDUCTORS AS SHOWN IN EXISTING 6" CONDUITS IN DUCTBANK FROM MANHOLE PMH-3 TO MANHOLE PMH-6.
- 6 PROVIDE NEW CONDUCTORS AS DENOTED IN EXISTING 6" CONDUIT IN DUCTBANK FROM MANHOLE PMH-6 TO MANHOLE PMH-5.
- 7 PROVIDE EQUIPMENT GROUNDING FOR TRANSFORMERS. REFER TO SHEET E7.03-01 DETAILS 2,3,4 FOR GROUNDING DETAILS.
- 8 CORE DRILL EXISTING MANHOLE FOR CONDUIT ENTRY.
- 9 PROVIDE MODULAR SPLICE WITHIN PMH-5 TO SERVE TRANSFORMER TH-A AND TH-B. REFER TO DETAIL 6/E7.01-01 FOR MODULAR SPLICE INFORMATION.
- 10 PROVIDE MEDIUM VOLTAGE CABLE TAGS FOR ALL NEW AND EXISTING CABLES IN EXISTING MANHOLES. REFER TO 6/E7.01-01 FOR INFORMATION.
- 11 PROVIDE NEW FEEDER IN NEW CONDUIT DUCTBANK AS SHOWN. REFER TO SHEET E6.01-01 FOR FEEDER AND CONDUIT SCHEDULE. REFER TO 3/E7.02-01 FOR DUCTBANK DETAILS.
- 12 PROVIDE NEW FEEDER IN NEW CONDUIT DUCTBANK AS SHOWN. REFER TO SHEET E6.01-01 FOR FEEDER AND CONDUIT SCHEDULE. REFER TO 6/E7.02-01 FOR DUCTBANK DETAILS.



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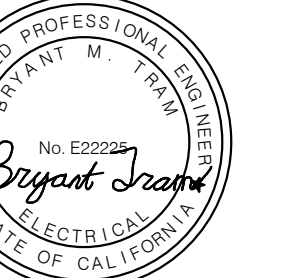
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CONSULTANTS



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PROJECT TITLE

COMPTON COLLEGE
STUDENT HOUSING
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD., COMPTON, CA 90221



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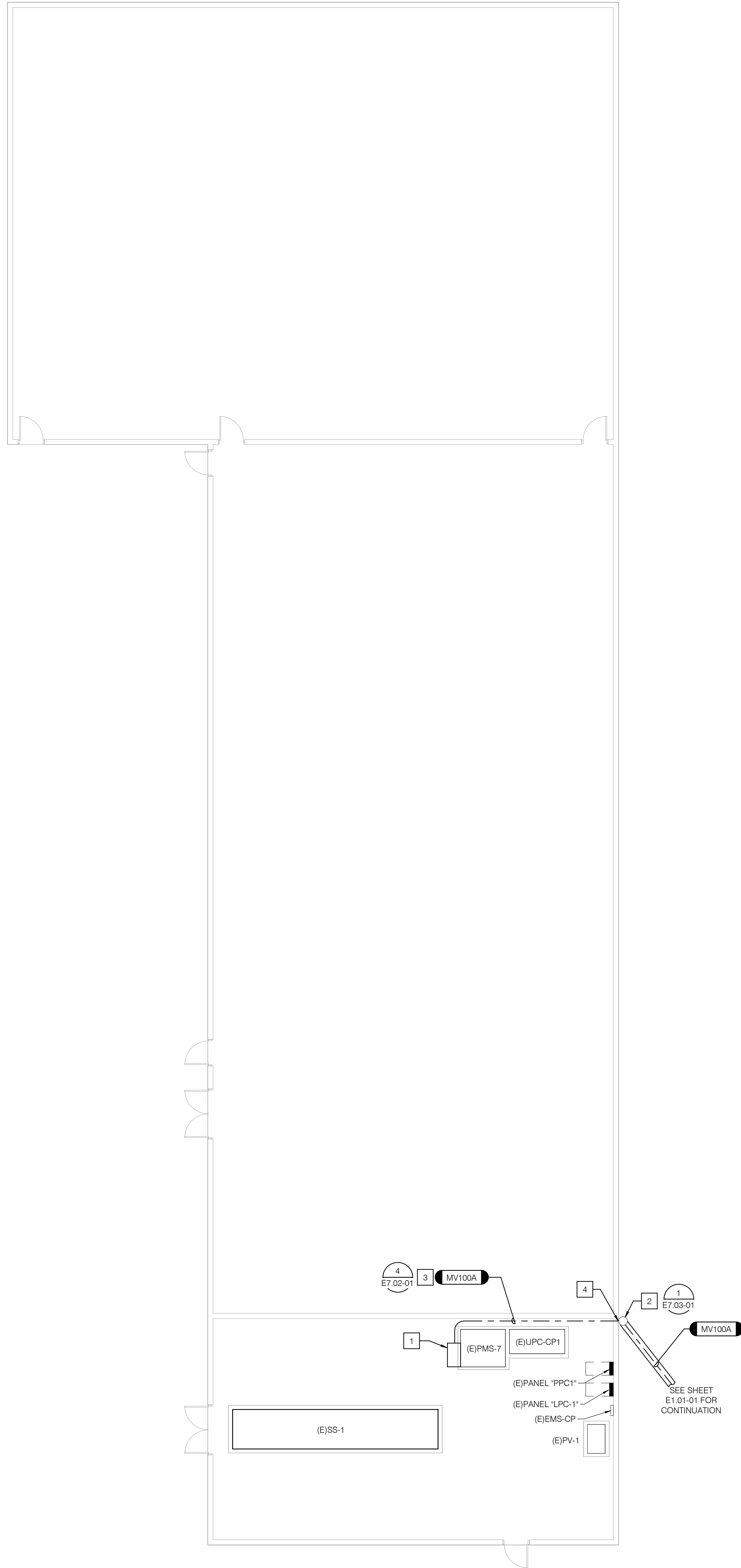
SHEET TITLE
SITE UTILITY PLAN

SHEET NUMBER

E1.01-01

CONSTRUCTION DOCUMENTS

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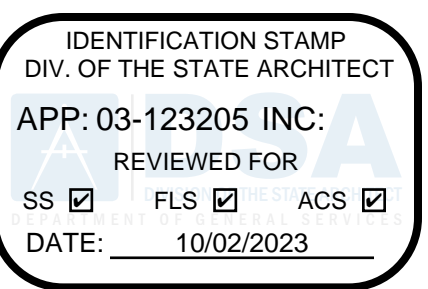
NOTES

- 1 PROVIDE PULLBOX 'PB-M2' MOUNTED ON SIDE OF EXISTING SWITCH PMS-7 ROUTING FOR NEW MEDIUM VOLTAGE FEEDERS. PULLBOX SHALL BE 48" H X 36" W X 48" D AND BE FITTED WITH REMOVEABLE COVERS.
- 2 PROVIDE LB FOR CONDUIT PENETRATION AND TRANSITION INTO AN UNDERGROUND TRENCH.
- 3 ROUTE NEW MV FEEDERS IN 4" CONDUIT MOUNTED ALONG CENTRAL PLANT WALL. REFER TO DETAIL 4/E7.02-01 FOR MOUNTING DETAIL.
- 4 CONDUIT TO HAVE A PENETRATION THROUGH THE CENTRAL PLANT WALL. EXISTING BEBAR TO BE TRACED AND MAKE NEW OPENINGS WITH MINIMUM 2" SEPERATION FROM REBAR.
- 5 PROVIDE NEW FEEDER IN NEW CONDUIT DUCTBANK AS SHOWN. REFER TO SHEET E6.01-01 FOR FEEDER AND CONDUIT SCHEDULE. REFER TO 2/E7.02-01 FOR DUCTBANK DETAILS.

GENERAL NOTES

- A. CAMPUS POINT OF SERVICE FOR MEDIUM VOLTAGE IS LOCATED AT SITE SWITCH 'PMS-7'.

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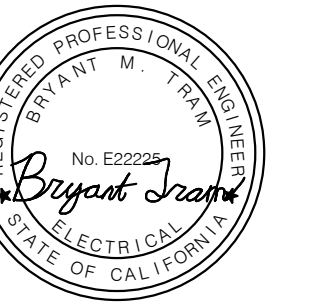


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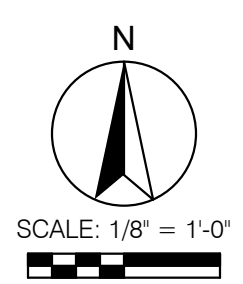
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SHEET TITLE
**CENTRAL PLANT
 BUILDING**

SHEET NUMBER

E2.10-01

CONSTRUCTION DOCUMENTS



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FEEDER AND CONDUIT SCHEDULE

Table with 3 columns: SYMBOLS, CONDUIT, SETS OF CONDUCTORS PER CONDUIT. Rows include 100A, 200A, 400A, 1600A, and 2000A.

FEEDER AND CONDUIT SCHEDULE

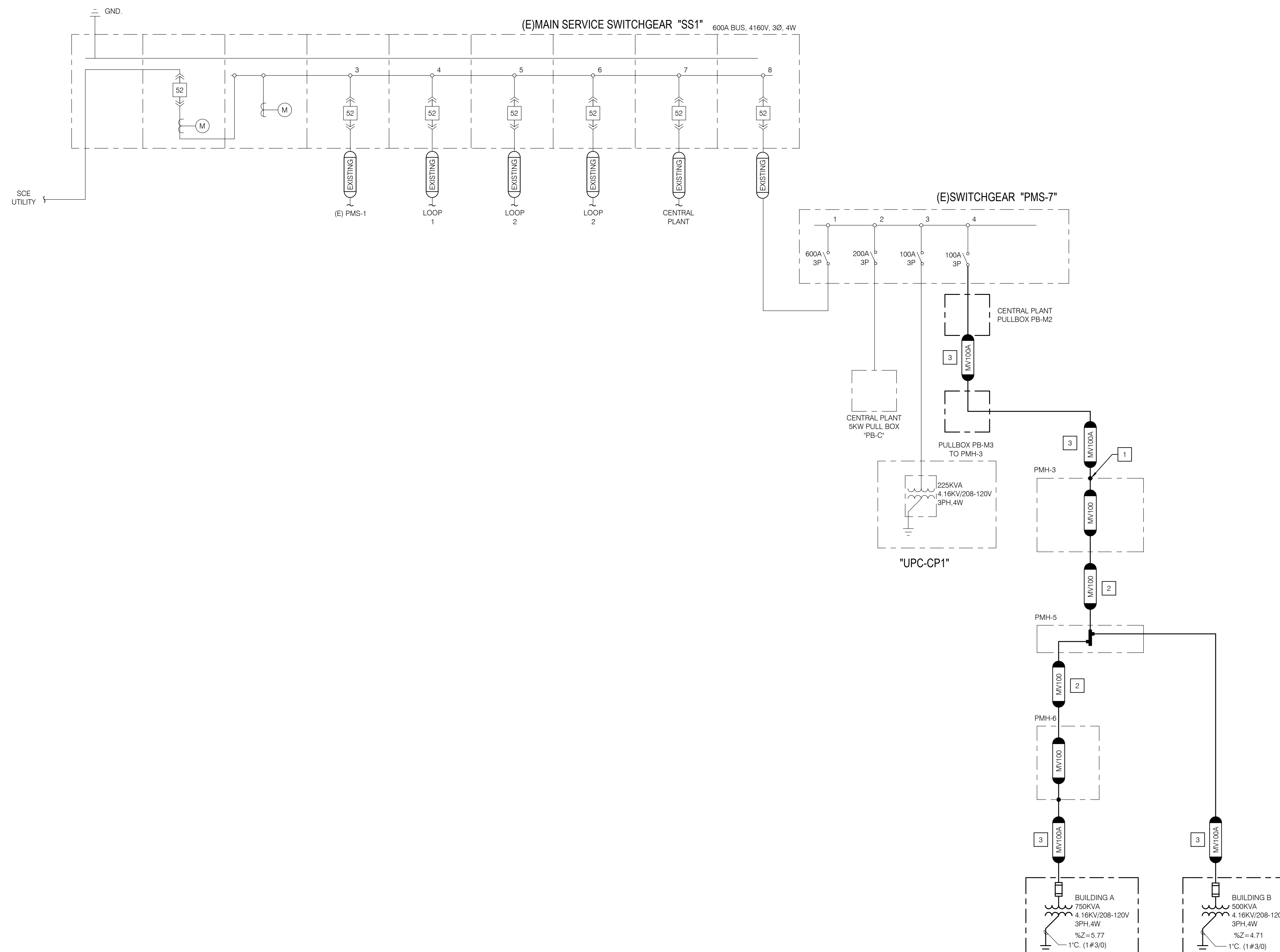
Table with 4 columns: SYMBOLS, CONDUIT, VOLTAGE, SETS OF CONDUCTORS PER CONDUIT. Rows include MV100A and MV100.

NOTES

- 1 CORE DRILL (E) MAN-HOLE FOR CONDUIT ENTRY.
2 PROVIDE AND PULL NEW CONDUCTORS IN EXISTING CONDUITS.
3 PROVIDE NEW FEEDER IN NEW CONDUIT DUCTBANK REFER TO SHEET E1-01-01. FOR SIZES REFER TO FEEDER AND CONDUIT SCHEDULE. REFER TO 2/E7-02-01 FOR DUCTBANK DETAILS.

GENERAL NOTES

- A NEW WORK IS SHOWN IN BOLD. ALL OTHER EQUIPMENT IS EXISTING TO REMAIN UNLESS OTHERWISE NOTED.
B ALL SWITCHGEAR SHALL BE ABB OR EQUAL BY EATON, SQUARE-D, OR SIEMENS.



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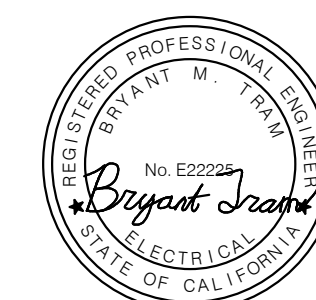
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ISSUED table with columns #, DATE, DESCRIPTION

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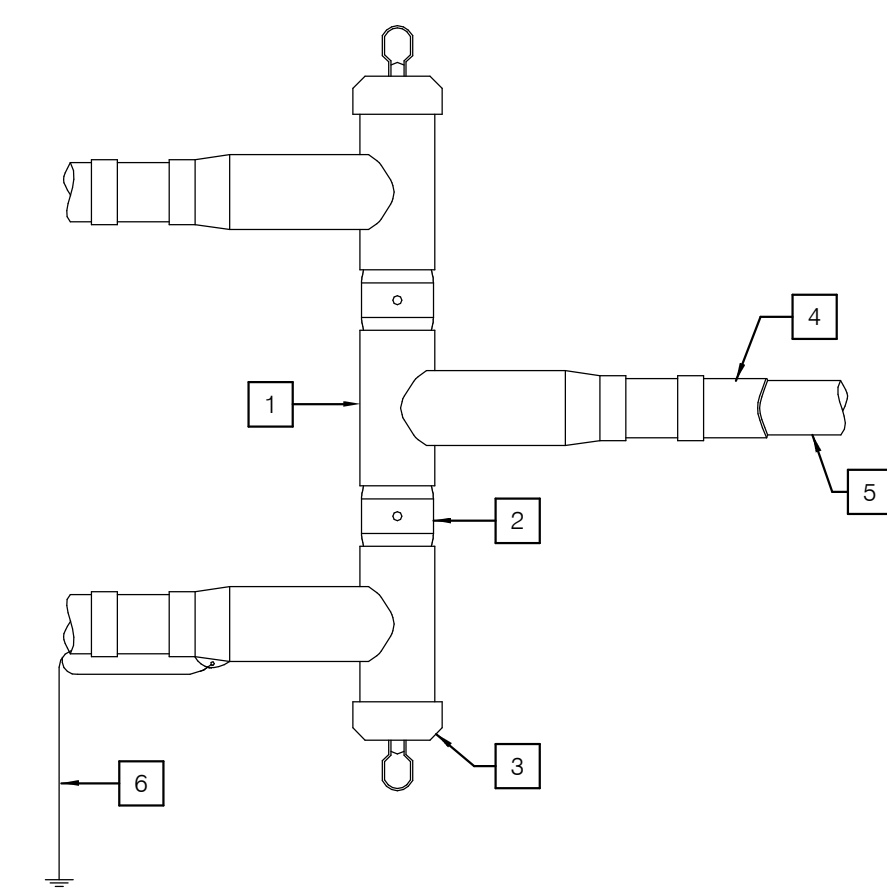
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SHEET TITLE
SINGLE LINE DIAGRAM-
MV UTILITY

SHEET NUMBER

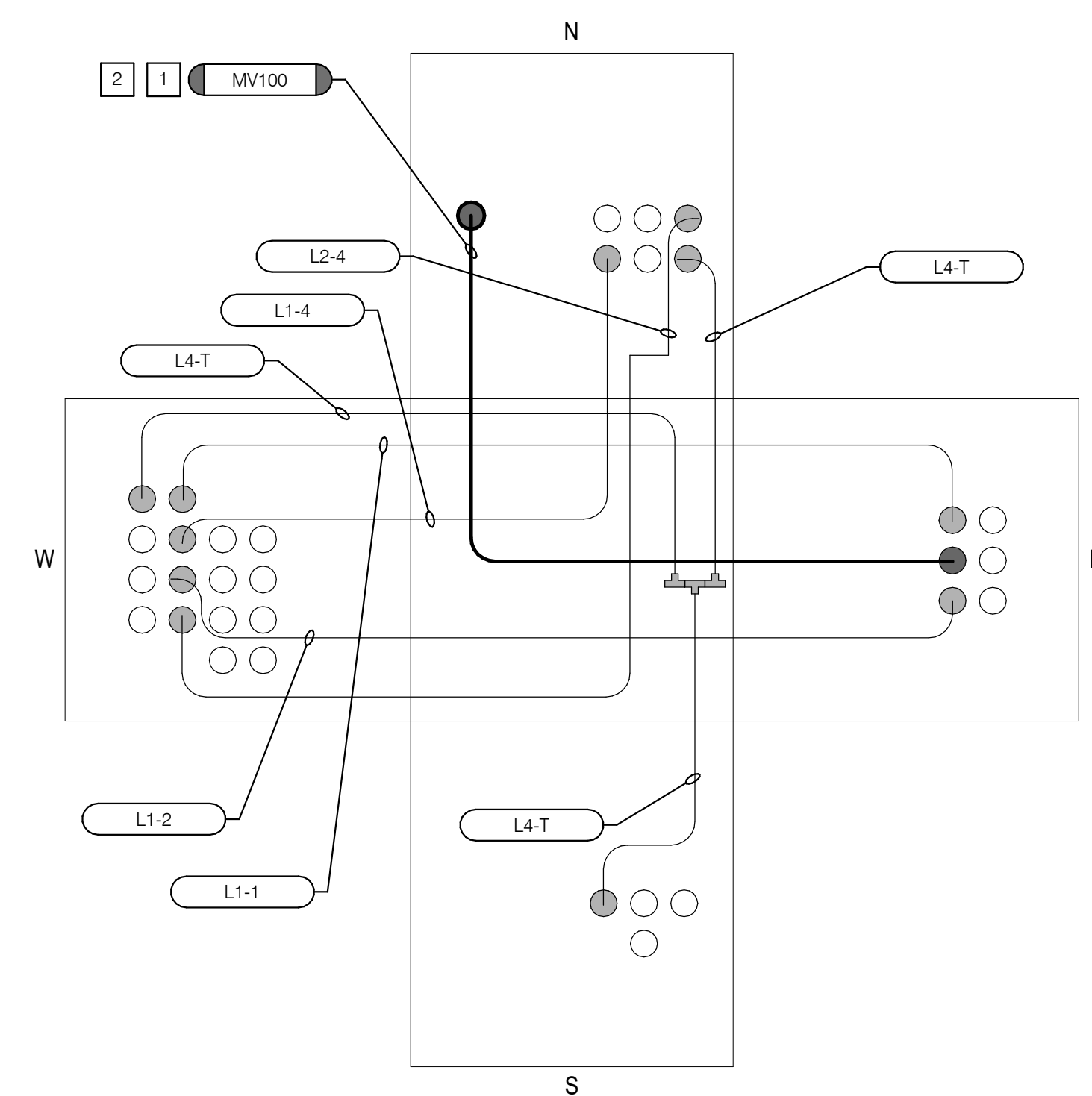
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CONSTRUCTION DOCUMENTS



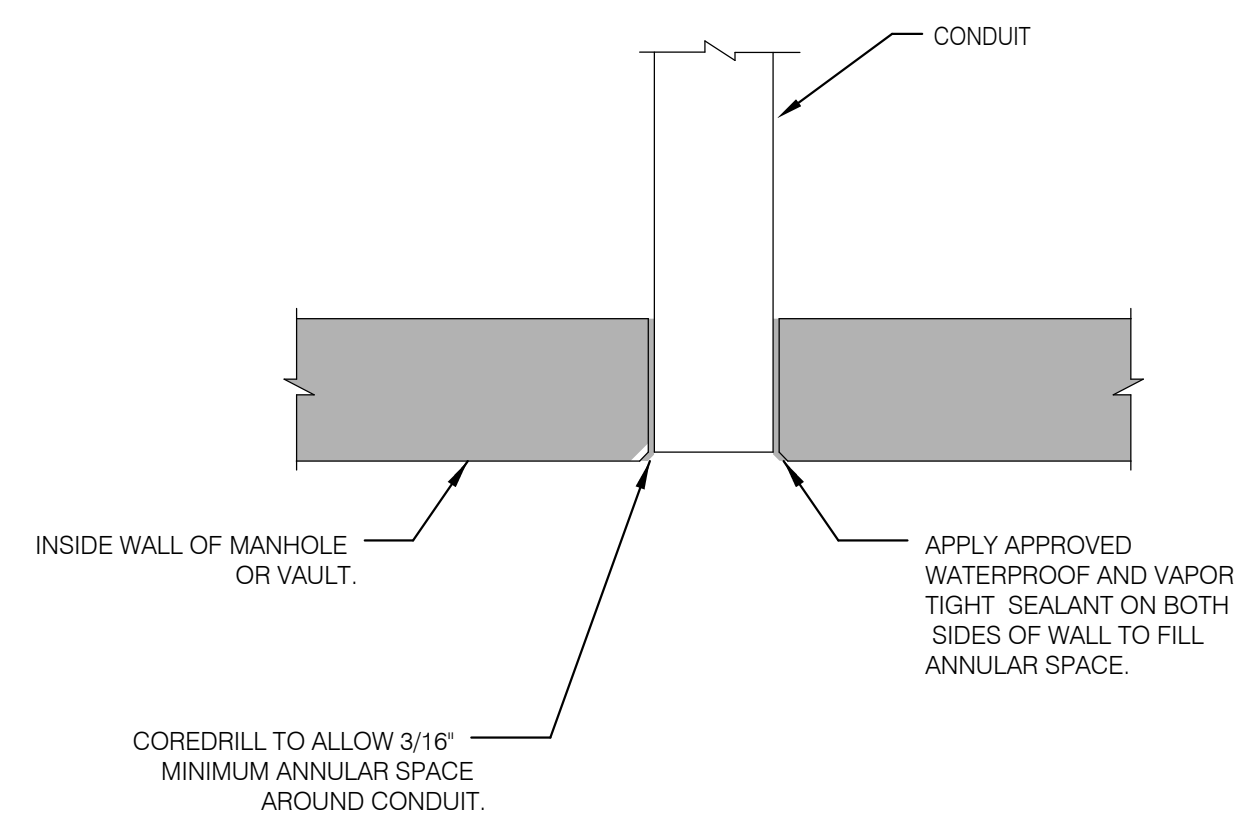
| | |
|---|--|
| 1 | 60A SEPARABLE, 15/25KV INSULATED DEAD-BREAK MODULAR CONNECTOR. |
| 2 | CONNECTING PLUG. |
| 3 | DEAD-END PLUG & VOLTAGE DETECTION CAP. |
| 4 | CABLE ADAPTOR KIT. |
| 5 | 5/8KV CABLE. |
| 6 | GROUNDING BRAID WITH MOISTURE BLOCK TO SYSTEM GROUND. |

6 MODULAR 3-WAY SPLICE
NO SCALE

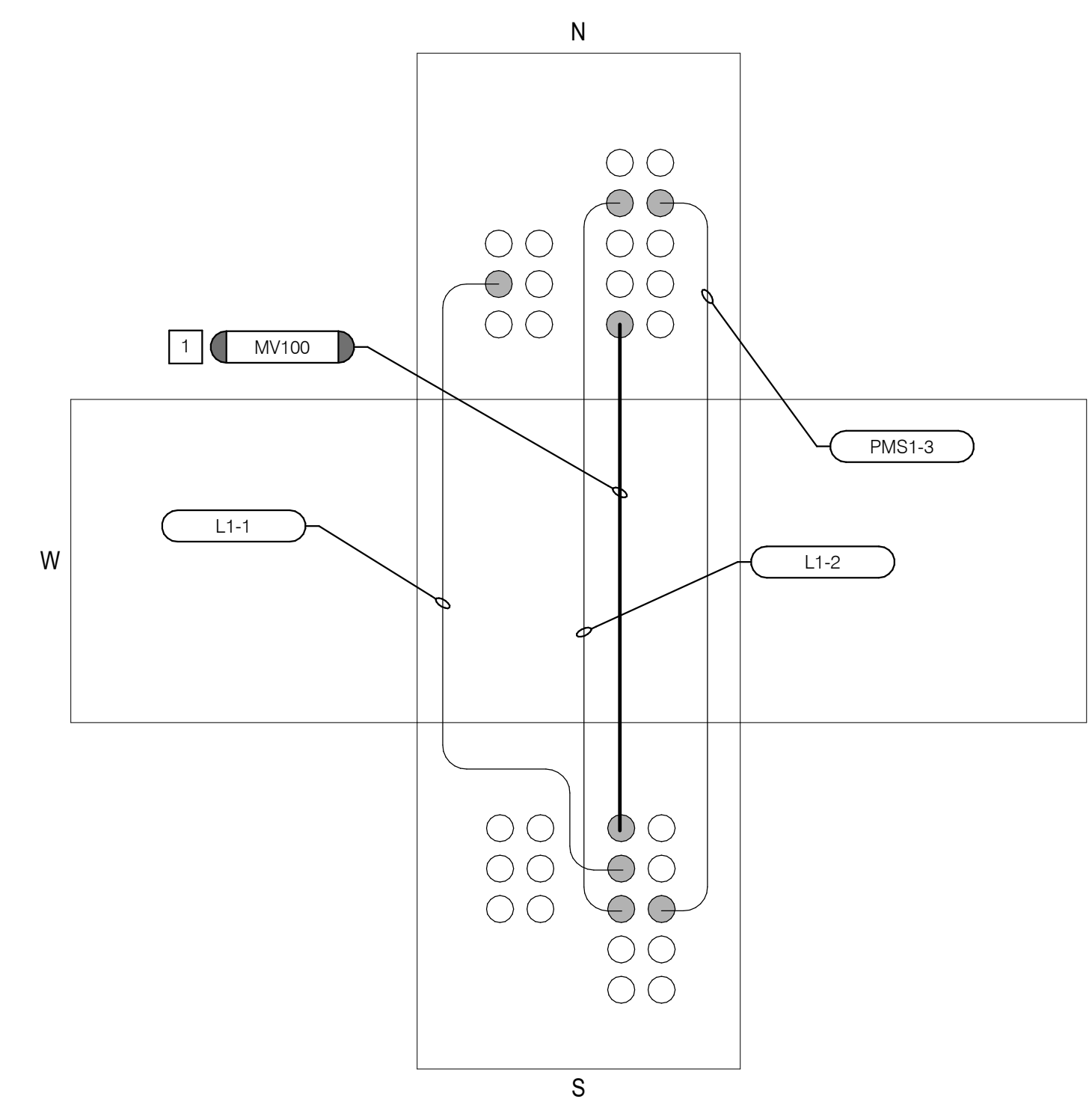


| | |
|---|---|
| 1 | CONTRACTOR TO CONFIRM EXACT LOCATION OF EXISTING CONDUIT TO ROUTE CONDUCTORS SHOWN FROM PMS-07 TO NEW PAD MOUNT TRANSFORMERS. |
| 2 | CORE DRILL EXISTING MANHOLE FOR CONDUIT ENTRY |

5 PMH-3 MANHOLE DIAGRAM
NO SCALE

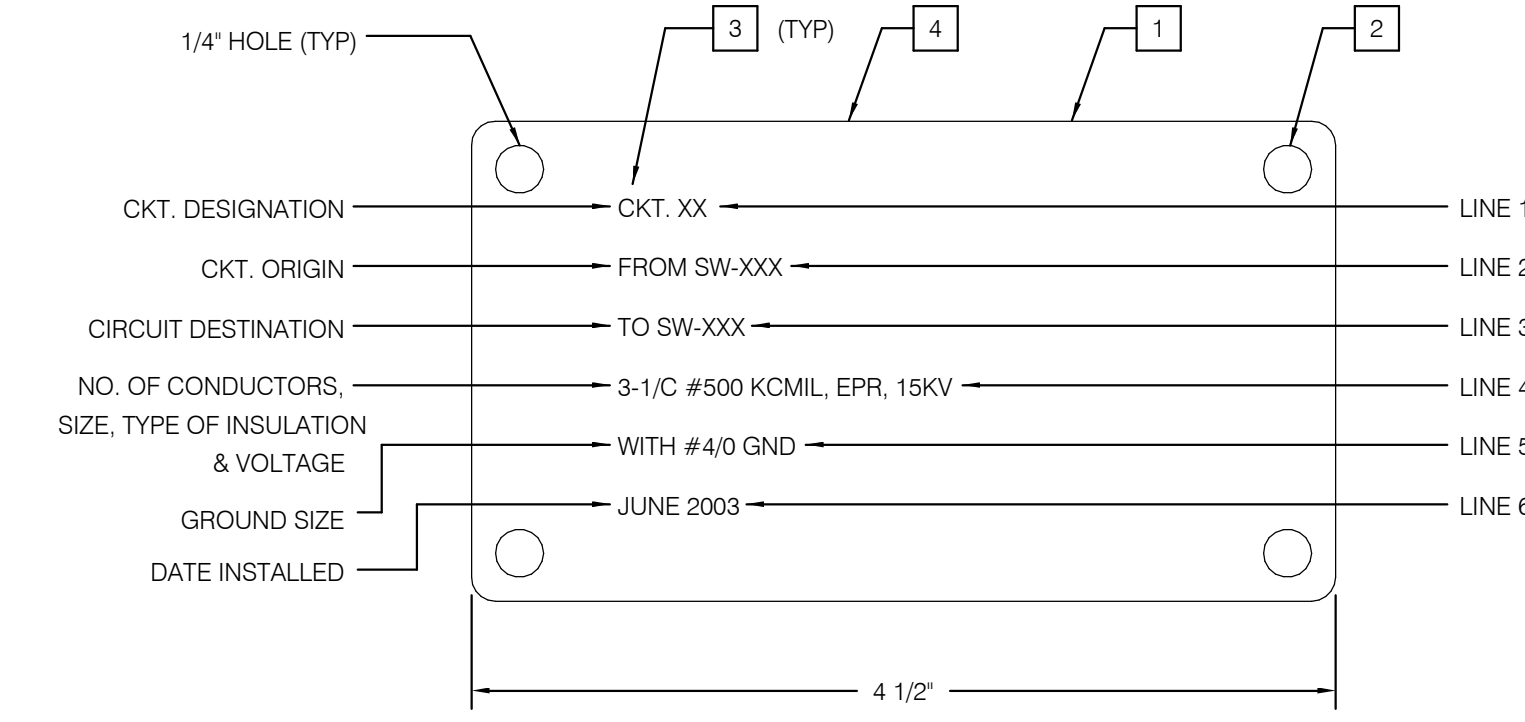


4 MANHOLE CORING
NO SCALE



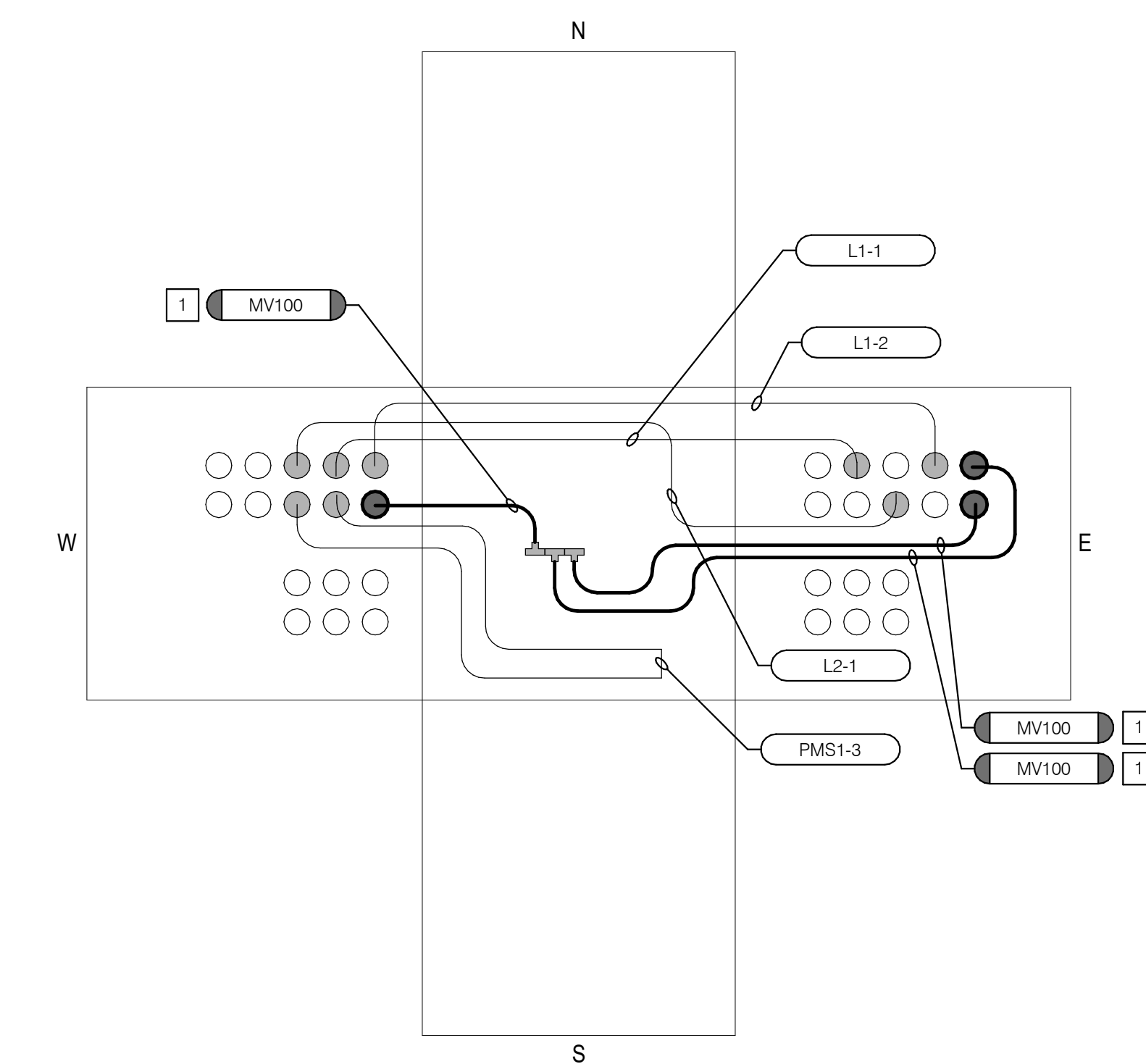
| | |
|---|---|
| 1 | CONTRACTOR TO CONFIRM EXACT LOCATION OF EXISTING CONDUIT TO ROUTE CONDUCTORS SHOWN FROM PMS-07 TO NEW PAD MOUNT TRANSFORMERS. |
|---|---|

3 PMH-6 MANHOLE DIAGRAM
NO SCALE



| | |
|---|---|
| 1 | 1/8" THICK LAMINATED WHITE MELAMINE (WITH BLACK CORE) PLASTIC TAG WITH MATTE WHITE FINISH, WITH CHARACTERS CUT THROUGH THE WHITE PLASTIC. |
| 2 | USE LOCK-ON TYPE NYLON TIES TO ATTACH TAG TO CABLE. |
| 3 | ALL LETTERS & NUMBERS ARE 1/4" HIGH. |
| 4 | FEEDER CIRCUITS IN EACH MANHOLE SHALL BE TAGGED. |

2 MEDIUM VOLTAGE CABLE TAG
NO SCALE



| | |
|---|---|
| 1 | CONTRACTOR TO CONFIRM EXACT LOCATION OF EXISTING CONDUIT TO ROUTE CONDUCTORS SHOWN FROM PMS-07 TO NEW PAD MOUNT TRANSFORMERS. |
|---|---|

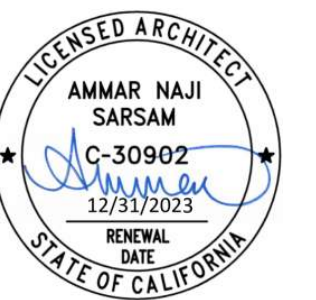
1 PMH-5 MANHOLE DIAGRAM
NO SCALE

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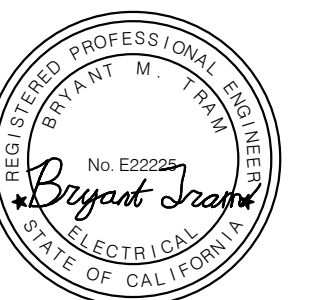


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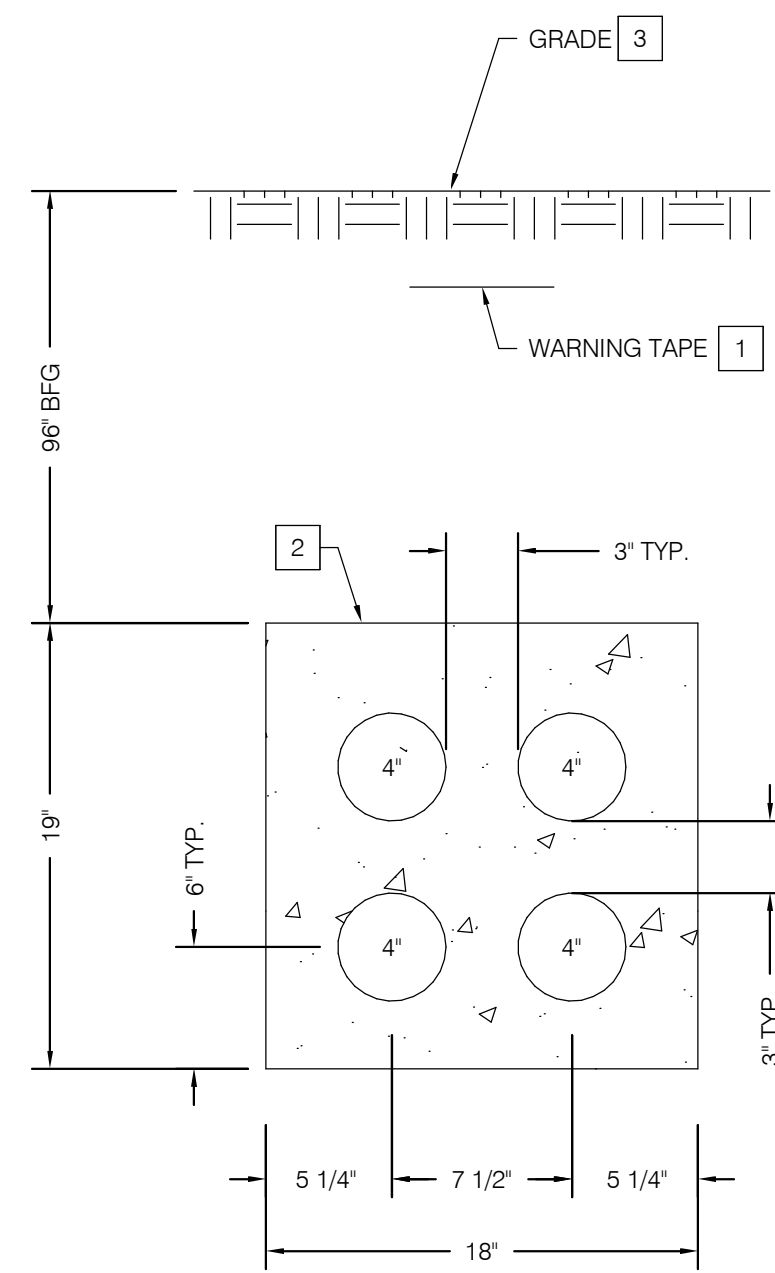
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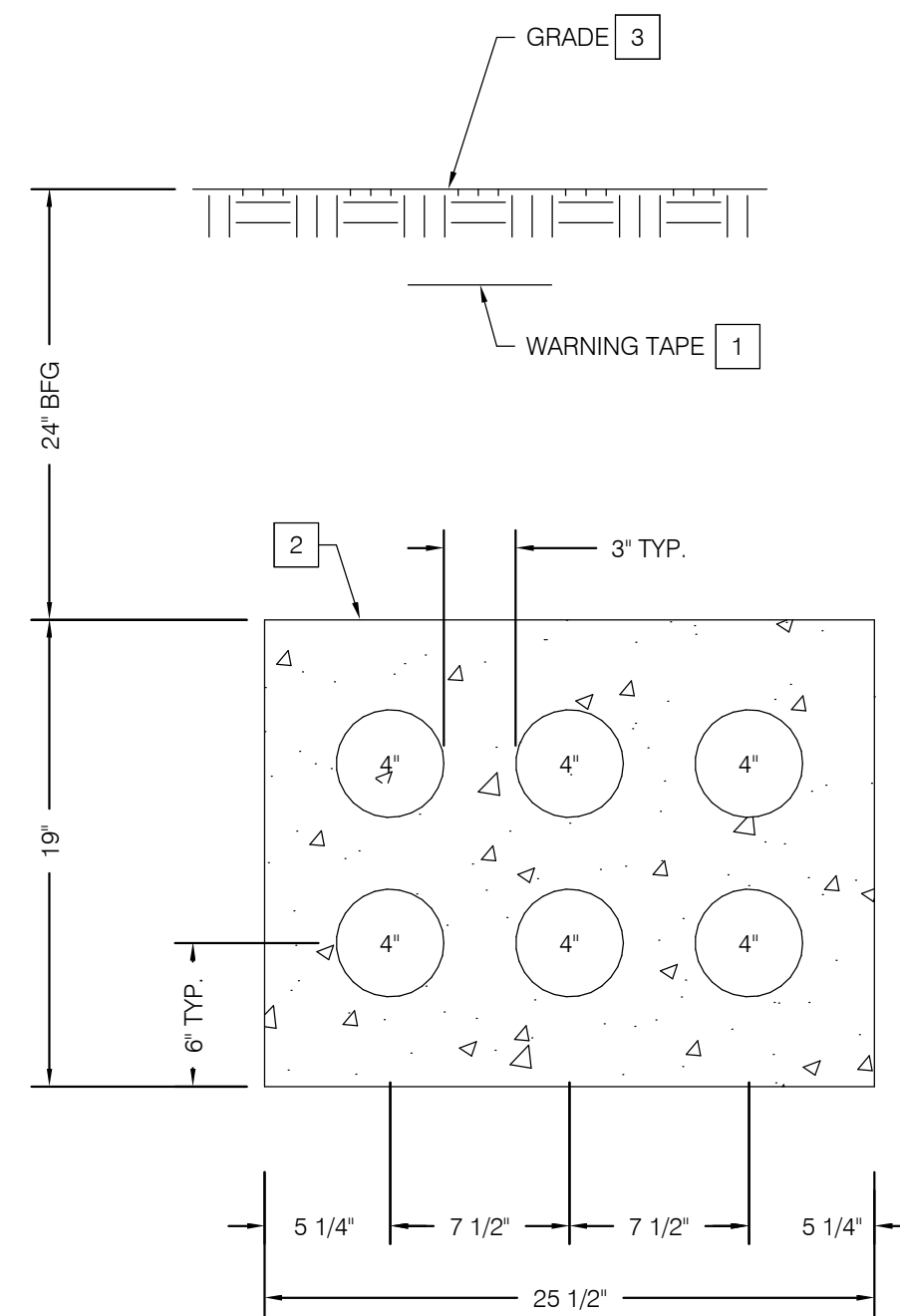
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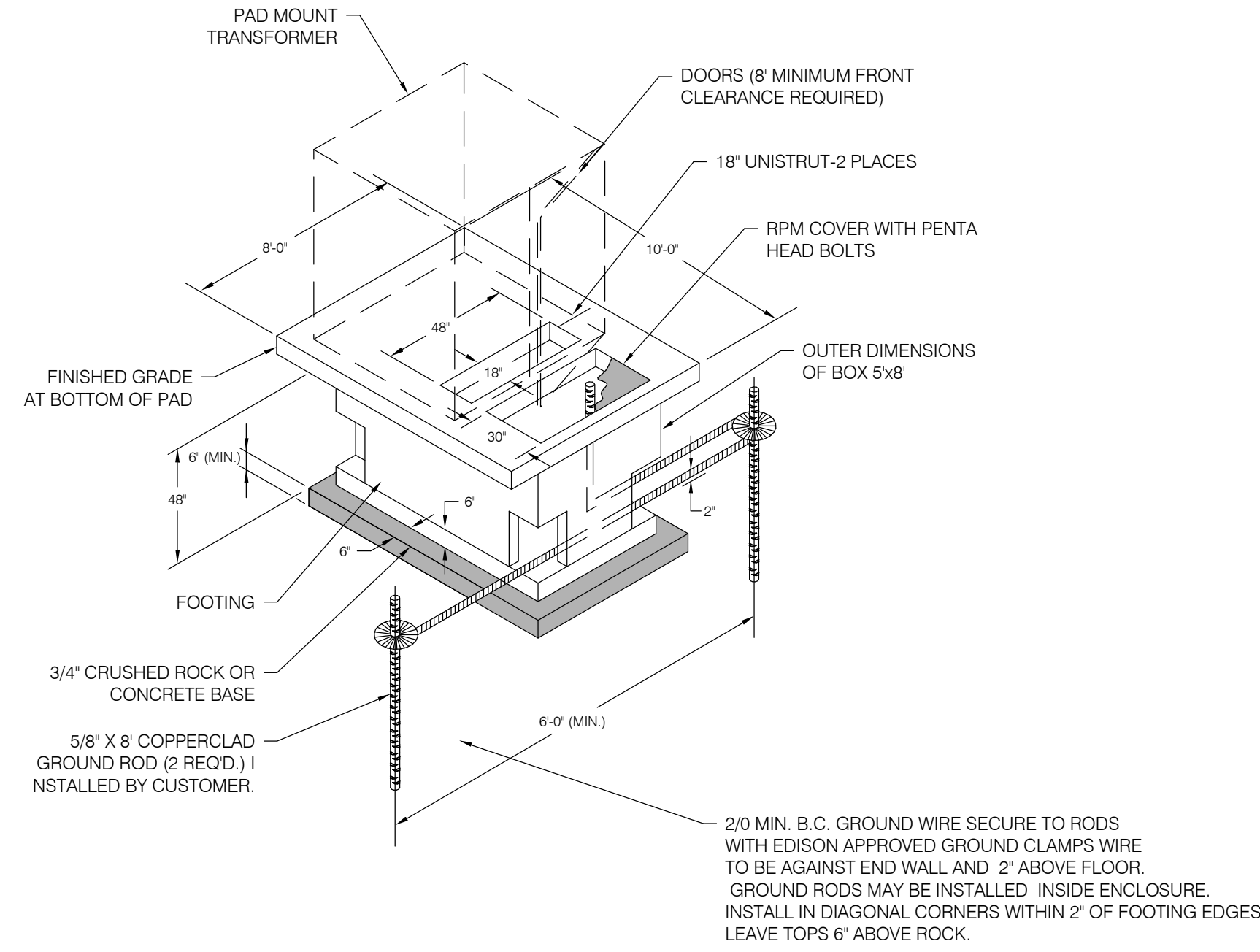
| NOTES | |
|-------|---|
| 1 | WARNING TAPE FOR MEDIUM VOLTAGE DUCTBANKS ONLY. PROVIDE AT 12" BELOW FINISH GRADE. |
| 2 | RED CONCRETE ENCASUREMENT CONSISTING OF 3 SACK OF CEMENT AND 10 LBS OF RED OXIDE (OR 1 GALLON OF RED COLOR) PER CUBIC YARD OF SAND FOR MEDIUM VOLTAGE DUCTBANKS ONLY. |
| 3 | RESTORE AREAS AND SURFACE AT AREAS DISTURBED TO MATCH EXISTING ADJACENT MATERIALS. REESTABLISH ORIGINAL GRADES. |

6 DUCTBANK BUILDING B - 4 CONDUITS
NO SCALE



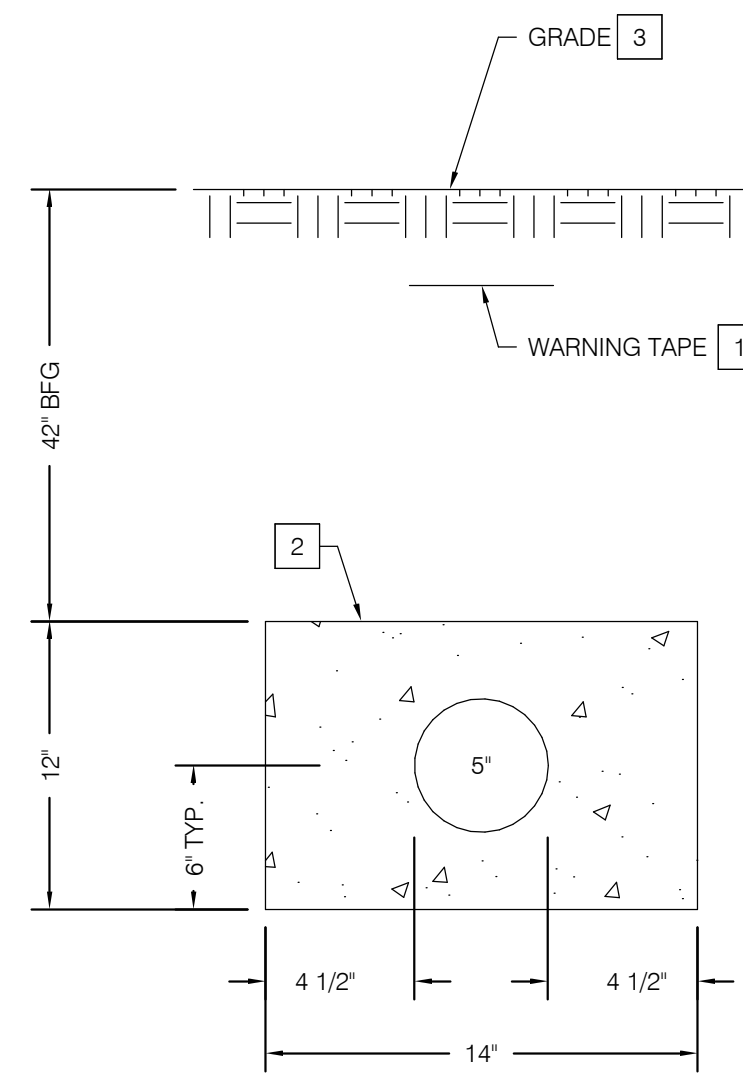
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| 3 | RESTORE AREAS AND SURFACE AT AREAS DISTURBED TO MATCH EXISTING ADJACENT MATERIALS. REESTABLISH ORIGINAL GRADES. |

5 DUCTBANK BUILDING A - 6 CONDUITS
NO SCALE



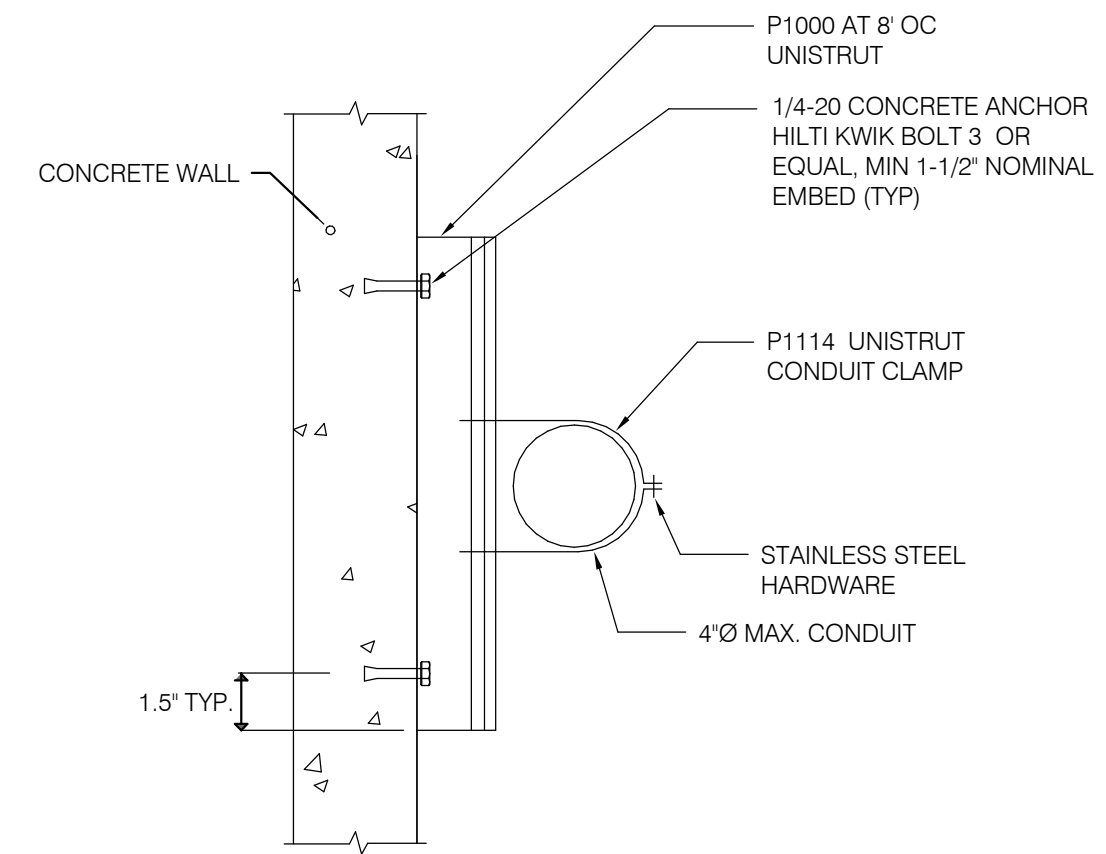
| NOTES | |
|-------|---|
| 1 | MASTIC REQUIRED AT JOINTS. |
| 2 | SIDE OR BACK OF PAD TO BE MINIMUM 3" FROM ADJACENT BUILDING SURFACE. MAY BE 2" IF BUILDING SURFACE IS NONCOMBUSTIBLE. |
| 3 | MINIMUM EXCAVATION INCLUDING ROCK 66" X 102" X 54" DEEP. |
| 4 | GROUNDING MATERIALS FURNISHED AND INSTALLED BY CONTRACTOR. |

3 TRANSFORMER PAD
NO SCALE



| NOTES | |
|-------|---|
| 1 | WARNING TAPE FOR MEDIUM VOLTAGE DUCTBANKS ONLY. PROVIDE AT 12" BELOW FINISH GRADE. |
| 2 | RED CONCRETE ENCASUREMENT CONSISTING OF 3 SACK OF CEMENT AND 10 LBS OF RED OXIDE (OR 1 GALLON OF RED COLOR) PER CUBIC YARD OF SAND FOR MEDIUM VOLTAGE DUCTBANKS ONLY. |
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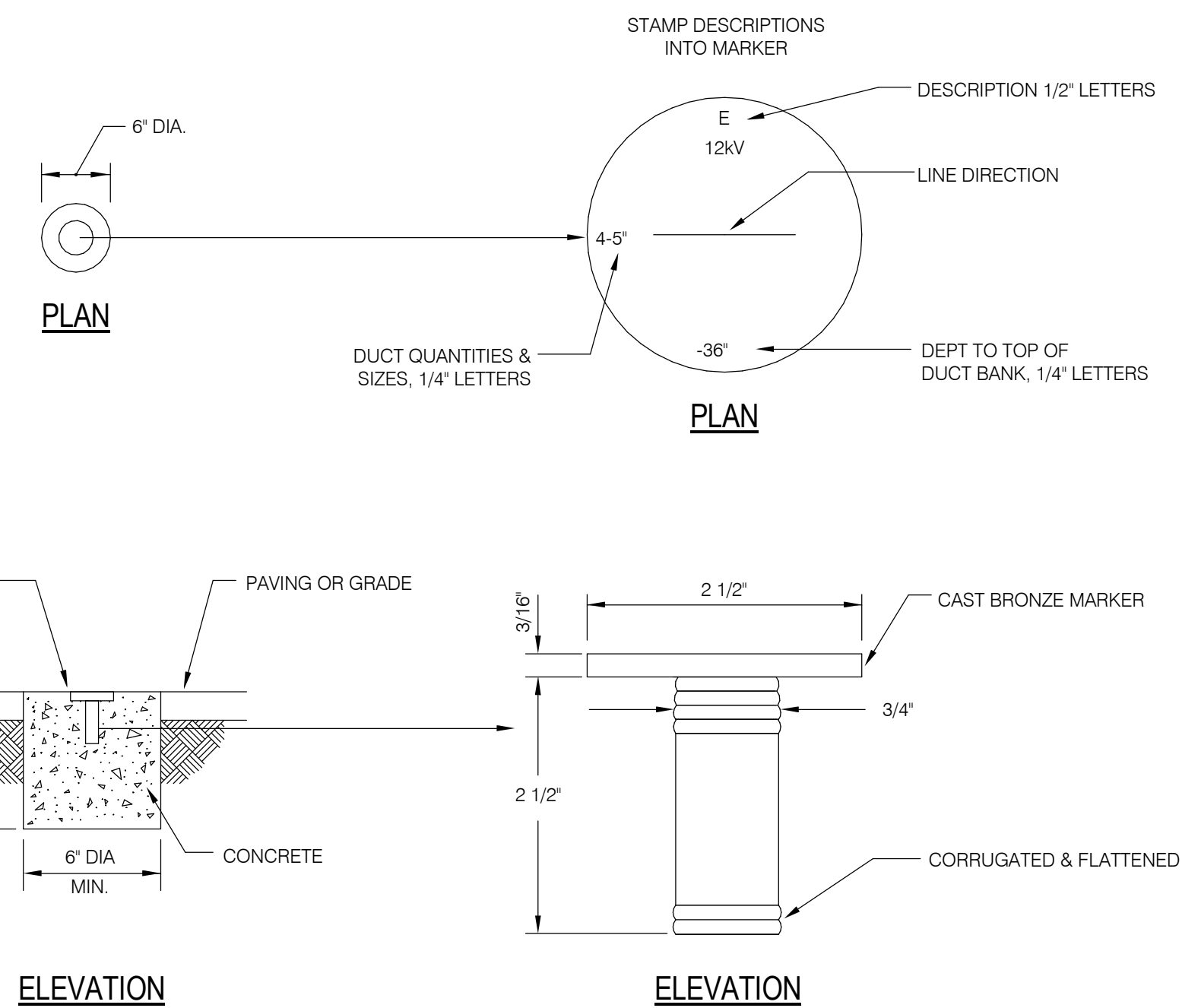
2 MV DUCTBANK
NO SCALE



GENERAL NOTES

- SPACING SHALL BE 6 FEET MAXIMUM.

4 SURFACE MOUNTED CONDUIT ON CONCRETE WALL
NO SCALE



| NOTES | |
|-------|--|
| 1 | THE TOP OF EACH MARKER SHALL BE MACHINED FLAT READY FOR STEEL STAMPING OR ENGRAVING, AND MAY HAVE A 45 DEGREE CHAMFER. |
| 2 | INSTALL A UTILITY MARKER AT THE FOLLOWING LOCATIONS: (A) CHANGE OF DIRECTION. (B) ALL BRANCH DUCTS. (C) EVERY 100' STRAIGHT RUN. (D) WHERE DUCTS ENTER A BUILDING. |

1 CONDUIT DUCT BANK - STUB OUT MARKER
NO SCALE

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DETAILS**

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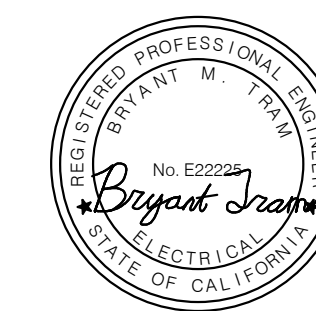


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 INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
 UNDERGROUND UTILITIES
 1111 E. ARTESIA BLVD., COMPTON, CA 90221



| ISSUED | |
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| # | DATE DESCRIPTION |
| 1 | 09/05/2023 DSA BACKCHECK SUBMITTAL |
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PROJECT IDENTIFICATION
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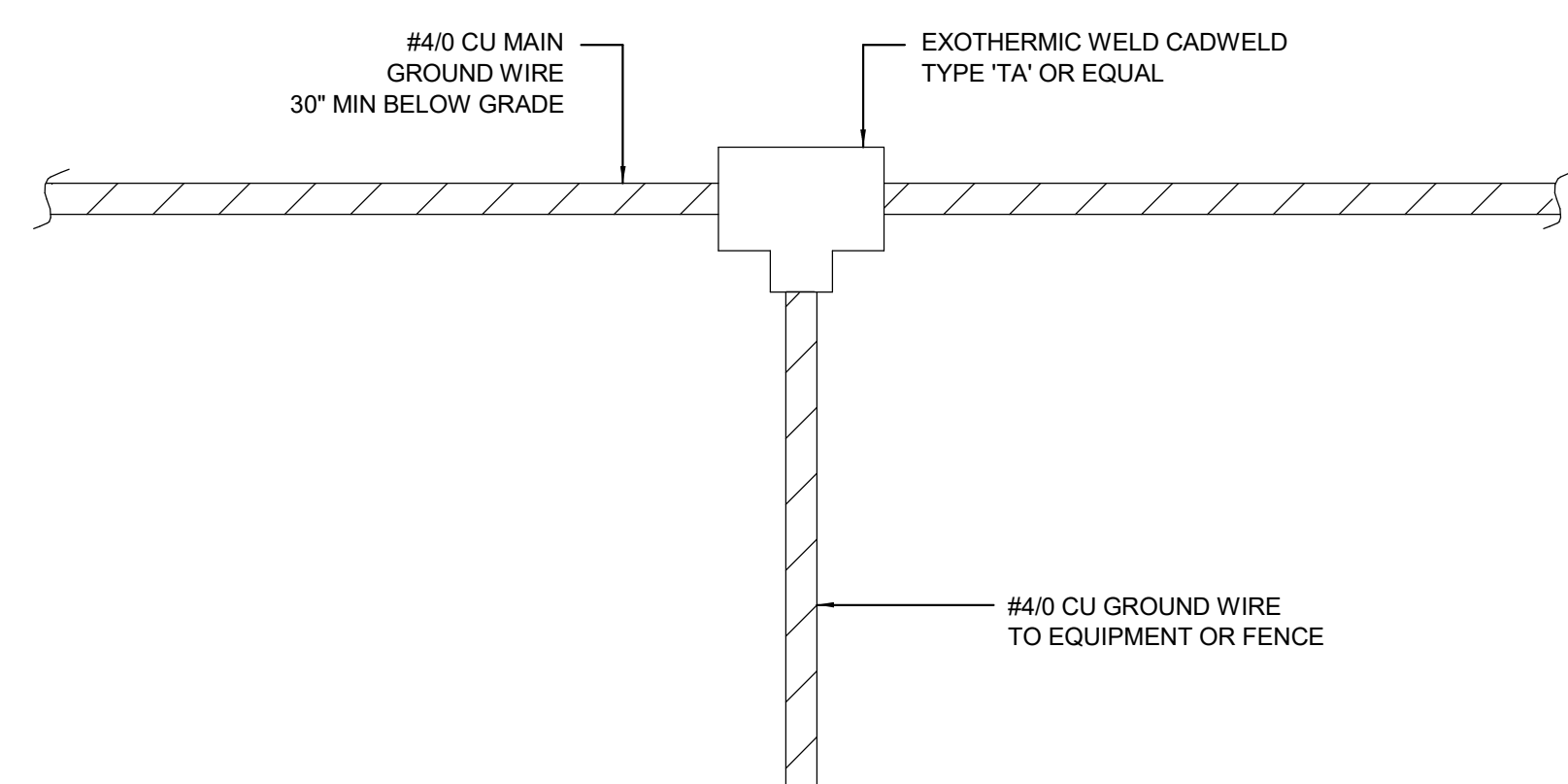
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SHEET TITLE
DETAILS

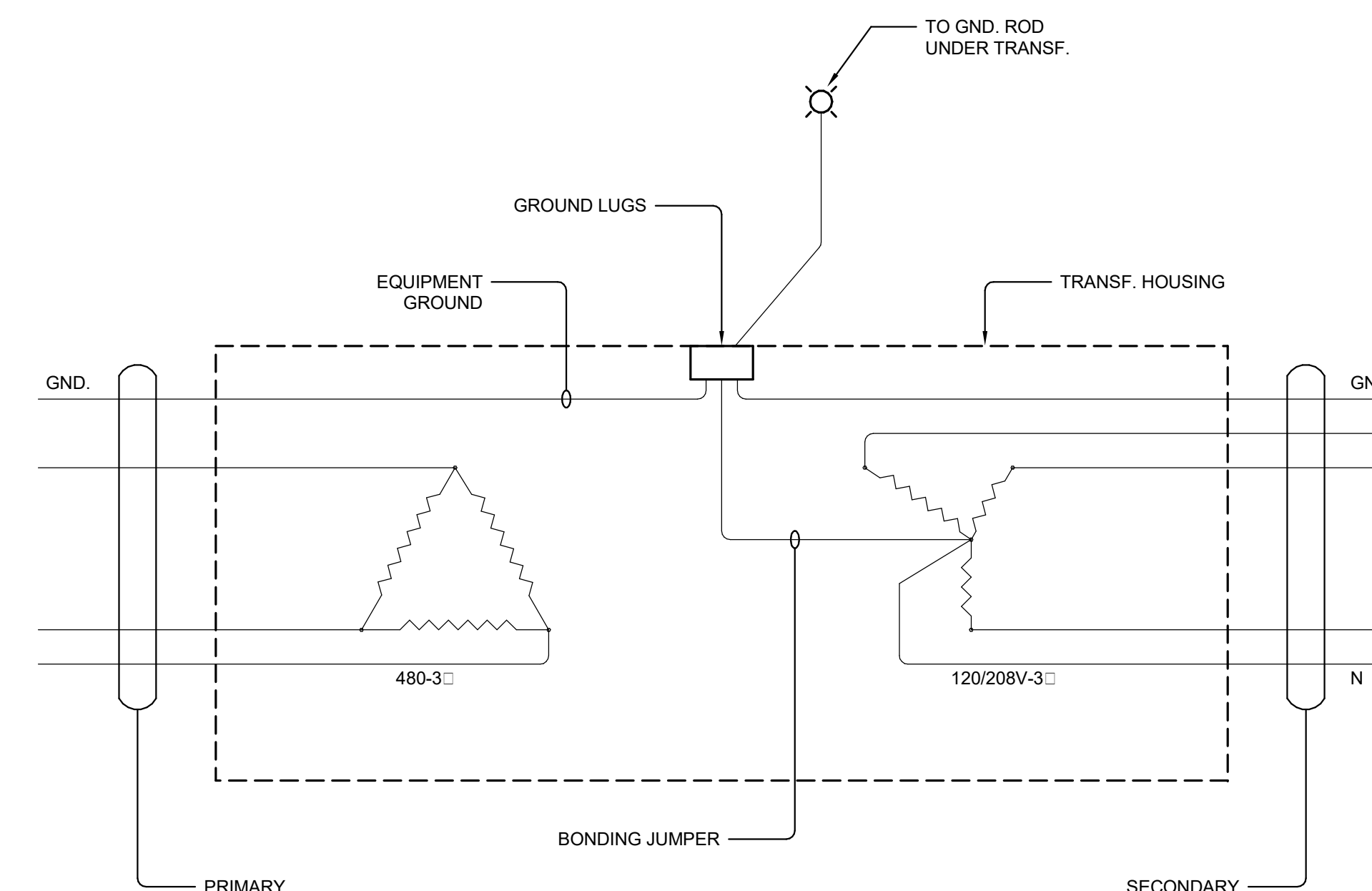
SHEET NUMBER

E7.03.01

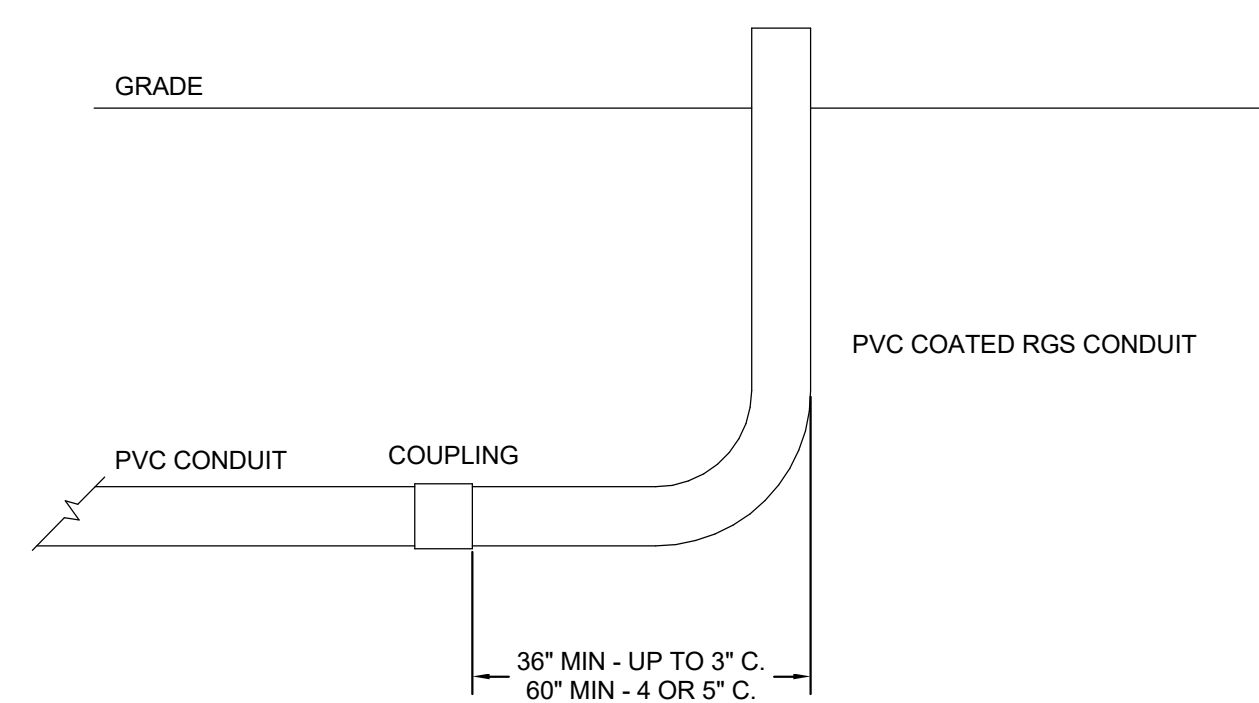
CONSTRUCTION DOCUMENTS



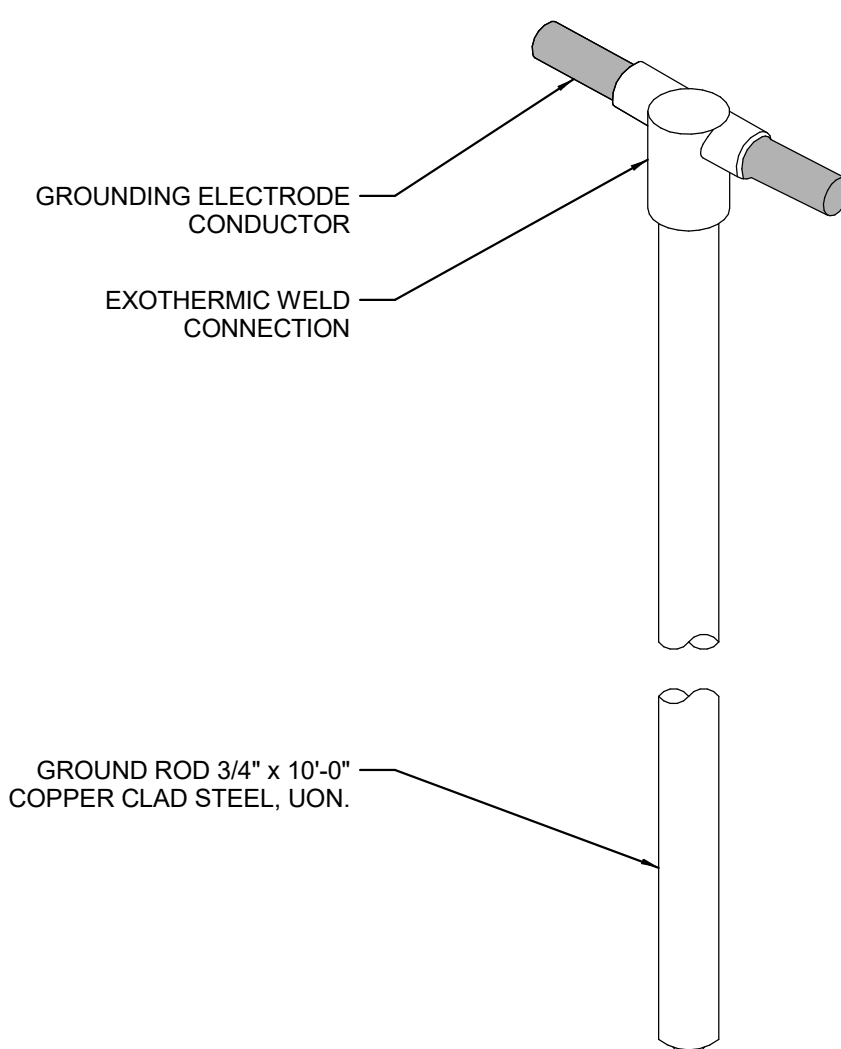
4 GROUND CONDUCTOR
 NO SCALE



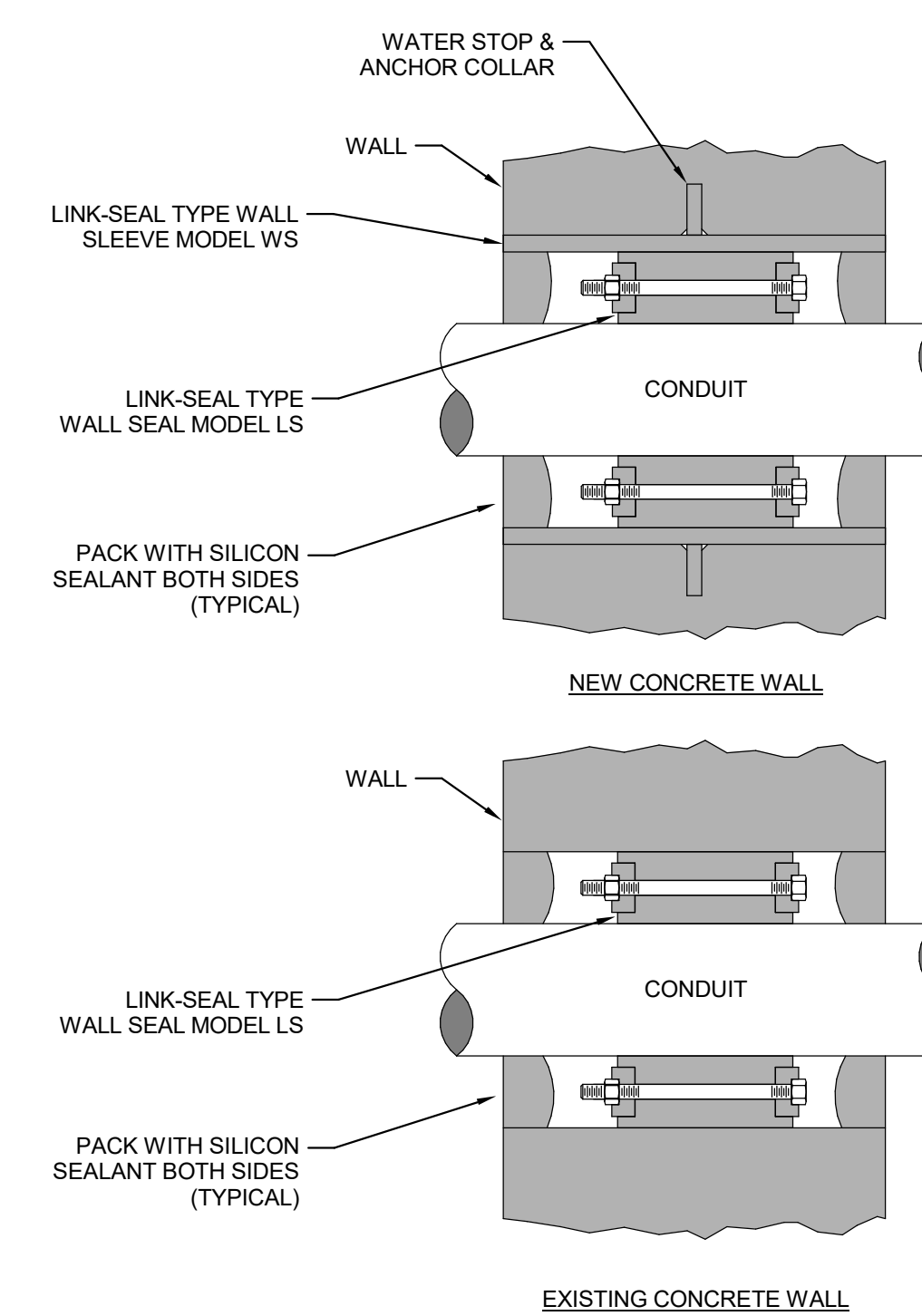
2 TRANSFORMER GROUNDING
 NO SCALE



5 CONDUIT RISER
 NO SCALE

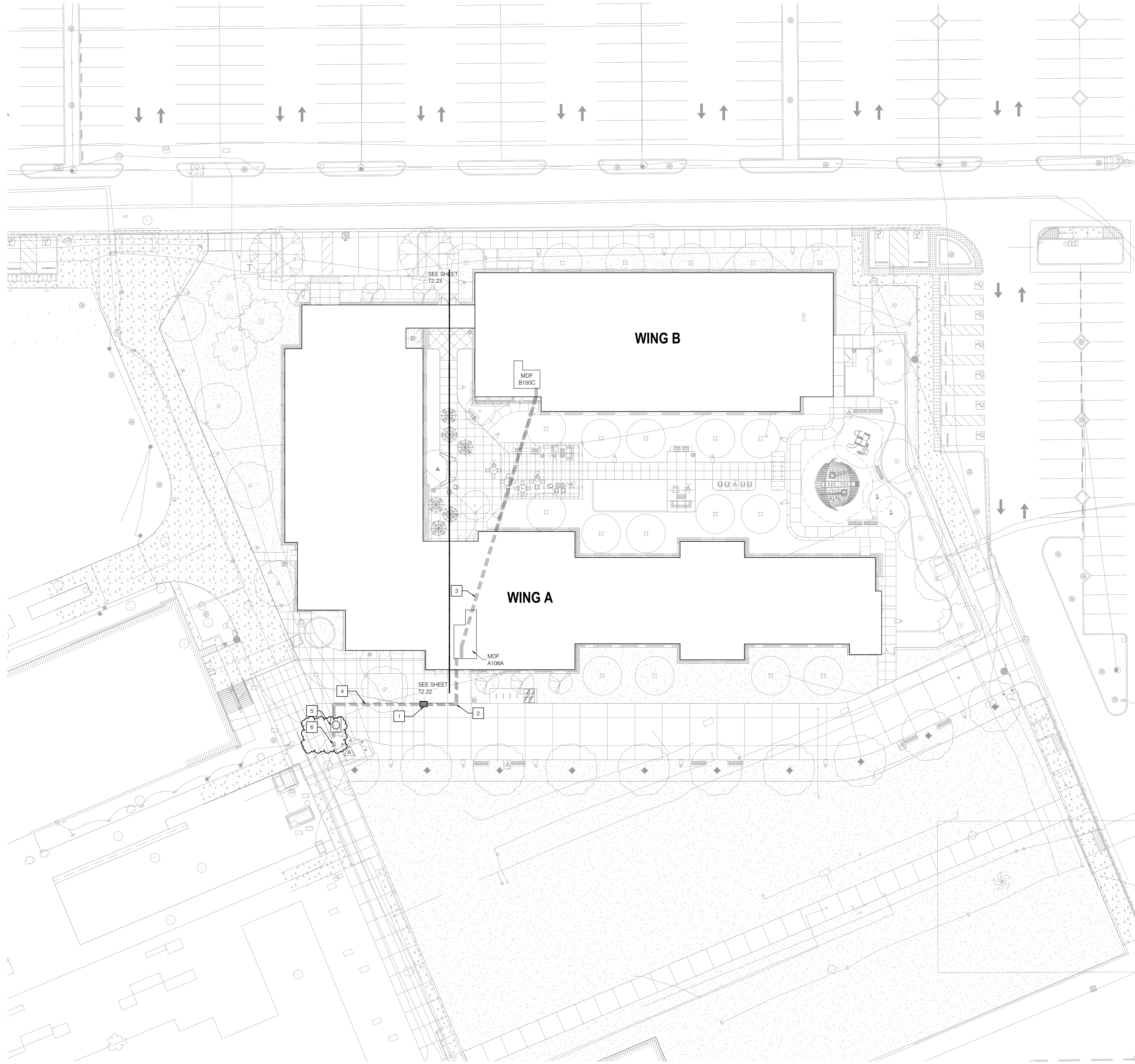


3 GROUND ROD
 NO SCALE



1 CONDUIT PENETRATION THRU CONCRETE WALL
 NO SCALE

3/18/2024 2:02:18 PM Autodesk Docs://Compton College Student Housing/Compton College Student Housing_AVTIS.rvt



NOTES

- 1 NEW 2X3 UNDERGROUND PULL BOX. REFER TO DETAIL 1/76.01-01.
- 2 NEW UNDERGROUND (4)4" PVC FROM NEW 2X3 UNDERGROUND PULL BOX STUB TO MDF A106A. REFER TO DETAIL 2/76.01-01.
- 3 NEW UNDERGROUND (4)4" PVC FROM MDF A106A STUB TO MDF B150C. REFER TO DETAIL 2/76.01-01.
- 4 NEW UNDERGROUND (4)4" PVC FROM EXISTING UNDERGROUND PULL BOX STUB TO NEW 2X3 UNDERGROUND PULLBOX. REFER TO DETAIL 2/76.01-01.
- 5 EXISTING COMMUNICATION UNDERGROUND PULLBOX.
- 6 EXISTING UNDERGROUND CONDUIT PATHWAY. PROVIDE CABLE TRAY UTILIZING THE EXISTING UNDERGROUND CONDUIT PATH FOR A DIRECT CONNECTION BETWEEN MDF A106A AND THE CAMPUS DATA CENTER IN THE MIS BUILDING.

DSA STAMP



www.hpiarchitecture.com
115 22nd street
Newport Beach, CA
92663
o: 949.675.6442

SEAL



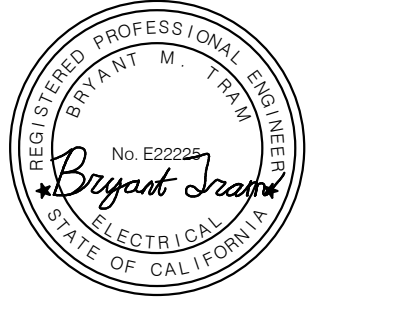
A# 03-123205 INC: 01

CONSULTANTS



Long Beach // Irvine // Los Angeles
San Diego // San Jose // Seattle

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PROJECT TITLE
**COMPTON COLLEGE
STUDENT HOUSING**
INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
UNDERGROUND UTILITIES
1111 E. ARTESIA BLVD., COMPTON, CA 90221



ISSUED

| # | DATE | DESCRIPTION |
|---|------------|-------------|
| A | 03/01/2024 | REVISION A |
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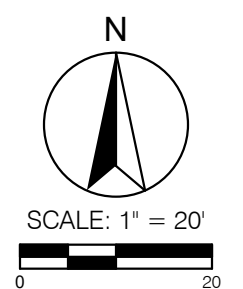
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SHEET TITLE
SITE PLAN

SHEET NUMBER

T1.01-01

CONSTRUCTION DOCUMENTS



8/18/2023 7:03:10 AM Autodesk Docs://Compton College Student Housing/Compton College Student Housing_AVTIS.rvt

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| DIV. OF THE STATE ARCHITECT | |
| APP: 03-123205 INC. | |
| REVIEWED FOR | ACCS |
| SS | FLS |
| DATE: | 10/02/2023 |



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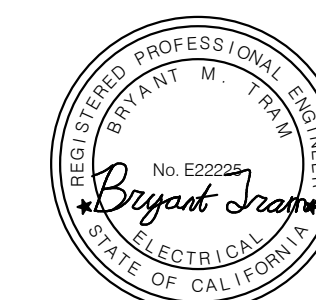


CONSULTANTS



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 San Diego // San Jose // Seattle

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PROJECT TITLE
**COMPTON COLLEGE
 STUDENT HOUSING**
 INCREMENT 1 OF 2 - DEMOLITION, EARTHWORK, &
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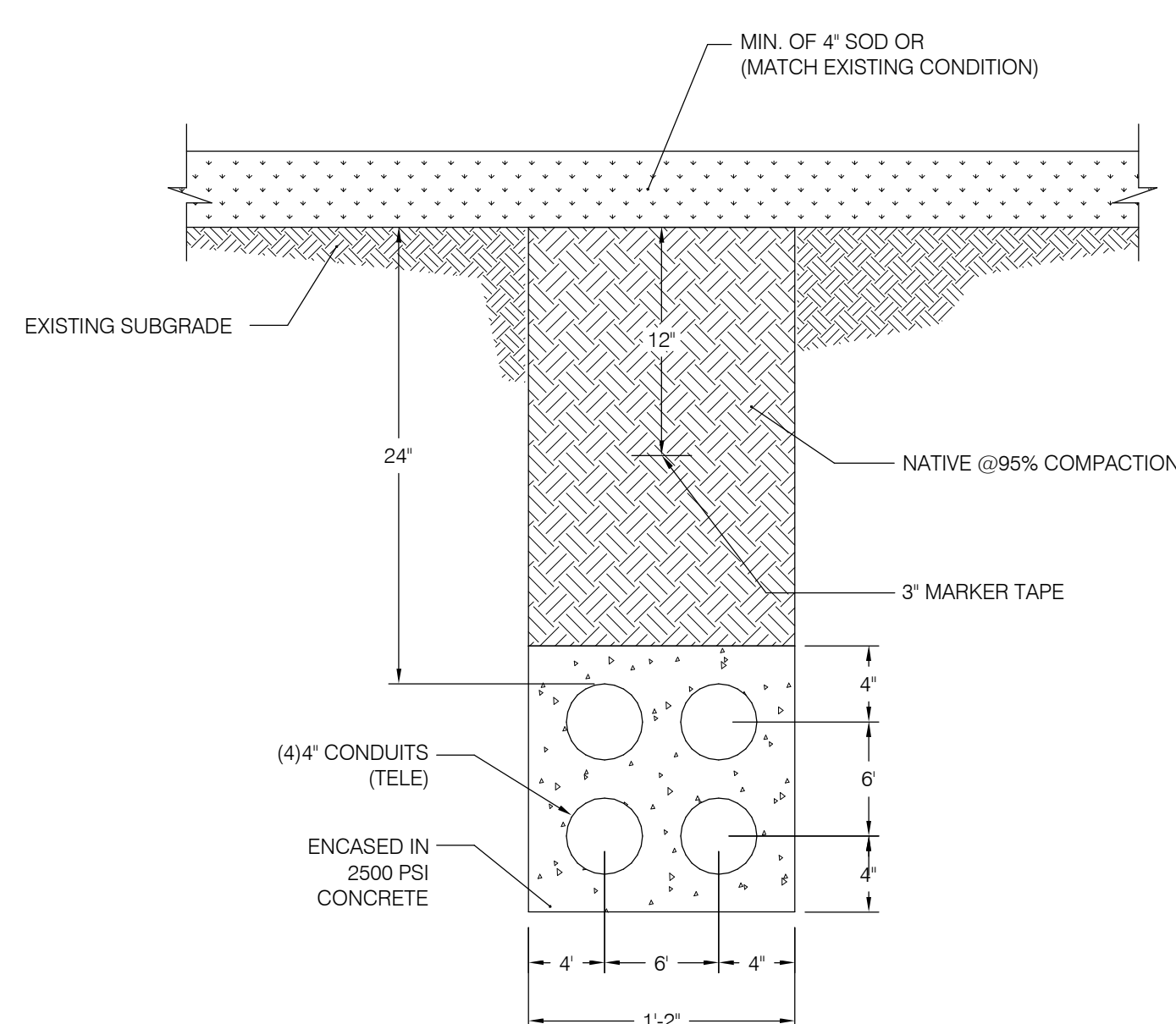
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SHEET TITLE
 DETAILS

SHEET NUMBER

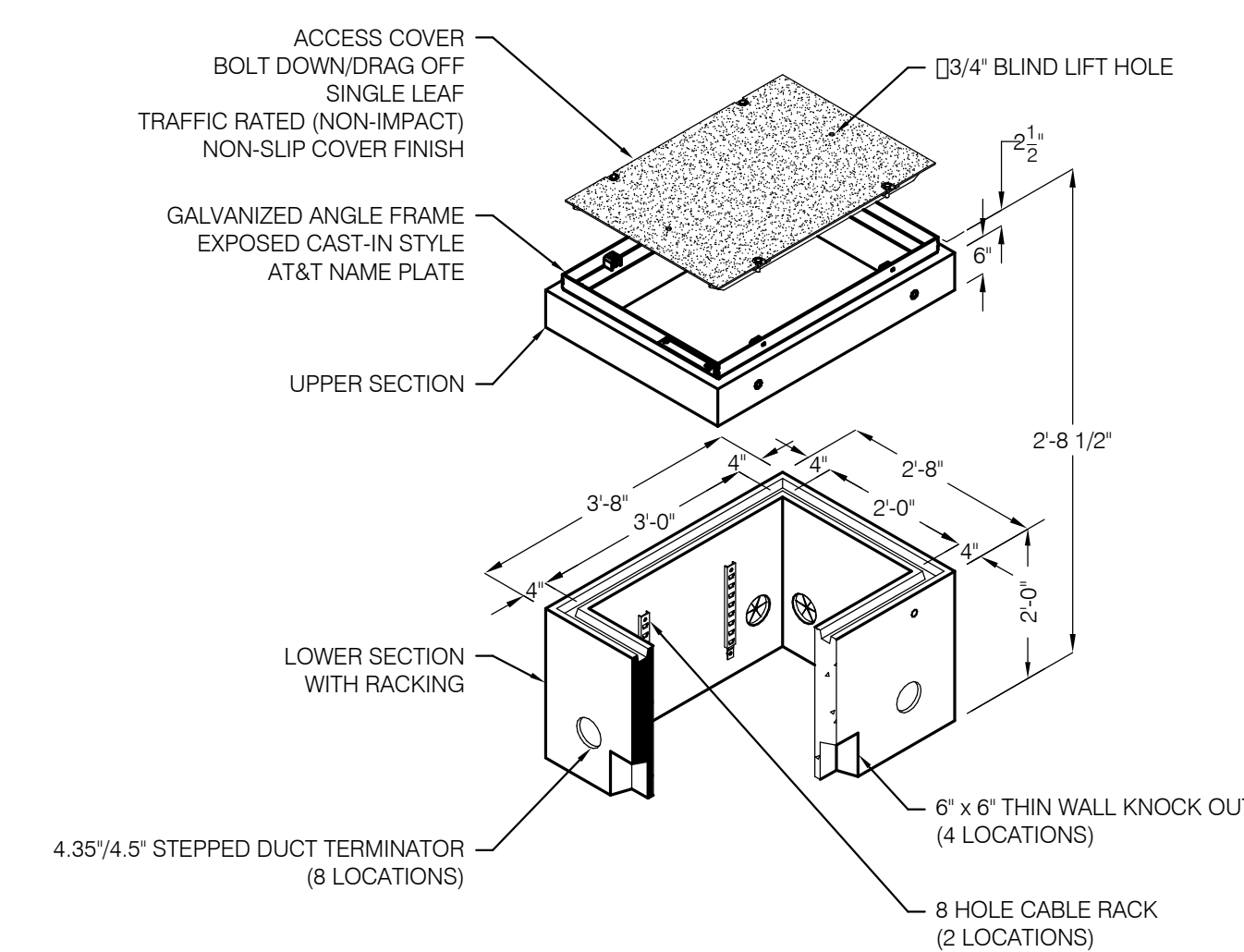
T6.01-01

CONSTRUCTION DOCUMENTS



- GENERAL NOTES**
- ALL PAVEMENT THICKNESSES ARE APPROXIMATE. CONTRACTOR SHALL MATCH EXISTING THICKNESS OR DIMENSIONS SHOWN, WHICHEVER IS GREATER.
 - APPLY TACK COAT PER SPECS. PROVIDE SMOOTH TRANSITION BETWEEN NEW AND EXISTING PAVEMENT SURFACES.

2 TRENCH - (4) 4" CONDUIT
 NO SCALE



1 2' x 3' PULLBOX
 NO SCALE